

# **DRAINAGE CALCULATIONS AND STORMWATER MANAGEMENT PLAN**

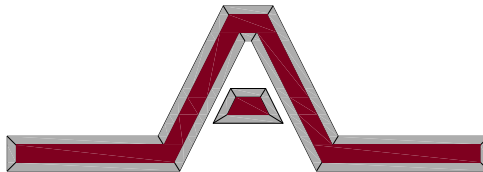
For The  
**Proposed Mixed-Use Development**

located at  
**39 & 41 Hillside Avenue  
(Tax Map 76 Lots 60 & 61)  
Amesbury, Massachusetts**

*Submitted to:*  
**City of Amesbury  
City Hall  
62 Friend Street  
Amesbury, MA 01913**

*Prepared for:*  
**Angiolillo Management Group Inc.  
99 Walnut Street  
Saugus, MA 01906**

*Prepared by*



**Engineering Alliance, Inc.**

Civil Engineering & Land Planning Consultants  
194 Central Street  
Saugus, MA 01906  
Tel: (781) 231-1349  
Fax: (781) 417-0020

1950 Lafayette Road  
Portsmouth, NH 03801  
Tel: (603) 610-7100  
Fax: (603) 610-7101



**October 26, 2021**



## **Table of Contents**

	Page
1. NARRATIVE	
▪ Project Description	1
▪ Site Description	1-2
▪ Pre-Development Condition	2
▪ Post Development Conditions	2
▪ Stormwater Management Facilities	3
▪ Erosion Control	3
▪ Figure 1 USGS Locus Map	1of5
▪ Figure 2 Ortho Photo	2of5
▪ Figure 3 FEMA	3of5
▪ Figure 4 Natural Heritage Map	4of5
▪ Figure 5 SCS Soils Map	5of5
○ SCS Soils Description	
2. APPENDICIES	
▪ APPENDIX A	Existing Conditions Drainage Calculations Existing Watershed Plan
▪ APPENDIX B	Proposed Conditions Drainage Calculations Proposed Watershed Plan
▪ APPENDIX C	BMP Operation and Maintenance Plan
▪ APPENDIX D	NOAA Rainfall Atlas



**Proposed Mixed-Use Development  
39 & 41 Hillside Avenue  
Amesbury, MA 01913**

**Project Description**

The project consists of the demolition of the existing structures (former automotive building and shed) and the construction of the proposed mixed-use building with driveway, parking area and landscaping. The subject property is comprised of approximately 23,717 S.F. of land and is located at 39 & 41 Hillside Avenue in Amesbury, MA (Tax Map 76 Lots 60 & 61). The property currently consists of a former automotive garage and shed with asphalt drive way and parking area.

The proposed project consists of the construction of the mixed used development with proposed bituminous concrete driveway and parking area, landscaped areas, incidental site grading, utility connection and stormwater management system.

The site abuts residential land to the north, west and south and Hillside Avenue to the east. Frontage and access are provided via Hillside Avenue.

**Site Description**

The subject property is currently occupied by a vacant automotive garage, a shed, asphalt parking area and landscaping. The topography of the site is general flat with slopes up to 4%. The site has a well-defined drainage pattern with storm runoff draining in a northeasterly direction towards Hillside Avenue. The majority of the site is comprised of impervious areas including the existing garage and asphalt parking area. The site does not contain any storm water management facilities in the pre-development condition. As a result, storm water runoff flows un-mitigated towards the design point.

In the proposed condition, the groundcover of the site will consist of the proposed mixed-use development, driveway and parking area and increased landscaped areas. In order to mitigate the stormwater runoff generated by the site, infiltration systems will constructed which will reduce the rate and volume of storm water runoff leaving the site while promoting groundwater recharge. The drainage patterns in the proposed condition will mimic those of the existing condition.

Soils information was obtained from available USDA Soil Conservation Service (SCS) Maps for Middlesex County. The soils on site are classified as Hinckley Loamy Sand (253B). Refer to Figure 5, SCS Soils Map, for a delineation of the boundaries of the soil with respect to the subject parcel and the attached SCS soil description. The soil conditions were confirmed by on-site soil testing performed on November 9, 2021.

The Flood Insurance Rate Map for the City of Amesbury (Community Panel 25009C0106F) with an effective date of July 3, 2012 describes the project as Zone X. Zone X is classified as areas determined to be outside the 0.2% chance floodplain.

All existing conditions information used has been compiled from an Actual on the ground survey prepared by Boston Survey, Inc. on March 20, 2021.

**Pre-Development Condition**

Technical Release 20 (TR-20) Program for Project Formulation Hydrology developed by the Soil Conservation Service (SCS) was employed to develop pre and post-development peak flows. Drainage calculations were performed for the pre-development condition for the 2, 10, 25, and 100-year type III 24-hour storm events. Refer to Appendix A for computer results, soil characteristics, cover descriptions and times of concentrations calculations.

In both the pre-development and post-development stormwater analysis two watershed areas

were analyzed. Refer to Existing Watershed Plan (EWP) in Appendix A for a delineation of the watershed areas as well as the location of the design points. The design points that were analyzed include design point #1 which is the closed drainage system on Hillside Avenue, design point #2 flows off-site to the northwest to Allencclair Drive.

A summary of the peak rates of the runoff during the Pre-Development Conditions is as follows:

**Pre-Development Condition Peak Discharge Summary (in CFS):**

	2-Year Storm (3.38 IN)	10-Year Storm (5.35 IN)	25-Year Storm (6.58 IN)	100-Year Storm (8.47 IN)
Design Point #1 (Hillside Ave)	1.01	1.88	2.41	3.23
Design Point #2 (Allencclair Drive)	0.33	0.59	0.75	1.00

**Proposed Development**

The proposed project consists of the construction of the mixed used development with proposed bituminous concrete driveway and parking area, landscaped areas, incidental site grading, utility connection and stormwater infiltration system. The increase in greenspace, alone, will result in a net decrease in the rate and volume of storm water runoff generated by the site. However in an effort to increase the quality of storm water runoff, a treatment stream for the parking area consisting of deep sump hooded catch basins to a water quality inlet to a sub-surface infiltration facility has been designed. The northerly parking area will also drain into its own subsurface infiltration system to mitigate stormwater runoff generated by the proposed parking area.

Proposed subsurface infiltration system #1 is comprised of 6 rows of 10 Cultec 330 XL HD Chambers. Stormwater generated by the proposed roof will drain via roof drains into the proposed system. Stormwater generated by the driveway and parking area will drain via surface flow into one of three catch basins which discharge into a Contech CDS water quality unit and ultimately into the proposed subsurface infiltration system. The northerly parking lot will drain via surface flow into a catch basin which discharges into a Contech CDS water quality unit and ultimately to proposed subsurface infiltrations system #2. This system is composed of 4 rows of 4 Cultec 330 XL HD Chambers. The systems have been sized based on the contributing areas while also incorporating an infiltration rate of 2.41 in/hr corresponding to the Rawls Rate established for a loamy sand type soil. The infiltration facilities will serve to reduce storm water runoff as well as promoting ground water recharge. The pervious paver walkway surrounding the northerly parking lot is comprise of approximately 718 square feet. The paver area is equipped with a choker course for water quality treatment followed by a 14-inch reservoir course that will store stormwater flows and allow for infiltration.

Again, drainage calculations were performed for the post-development condition for the 2, 10, 25, and 100-year type III 24-hour storm events. Refer to Appendix B for computer results, soil characteristics, cover descriptions, times of concentration calculations, and the Proposed Watershed Plans (PWP). A summary of the peak rates of runoff during the Post-Development Condition is as follows:

**Post-Development Condition Peak Discharge Summary (in CFS):**

	2-Year Storm (3.38 IN)	10-Year Storm (5.35 IN)	25-Year Storm (6.58 IN)	100-Year Storm (8.47 IN)
Design Point #1 (Hillside Ave)	0.00	0.02	0.07	0.16
Design Point #2 (Allencclair Drive)	0.00	0.00	0.01	0.04

**Stormwater Management Facilities**

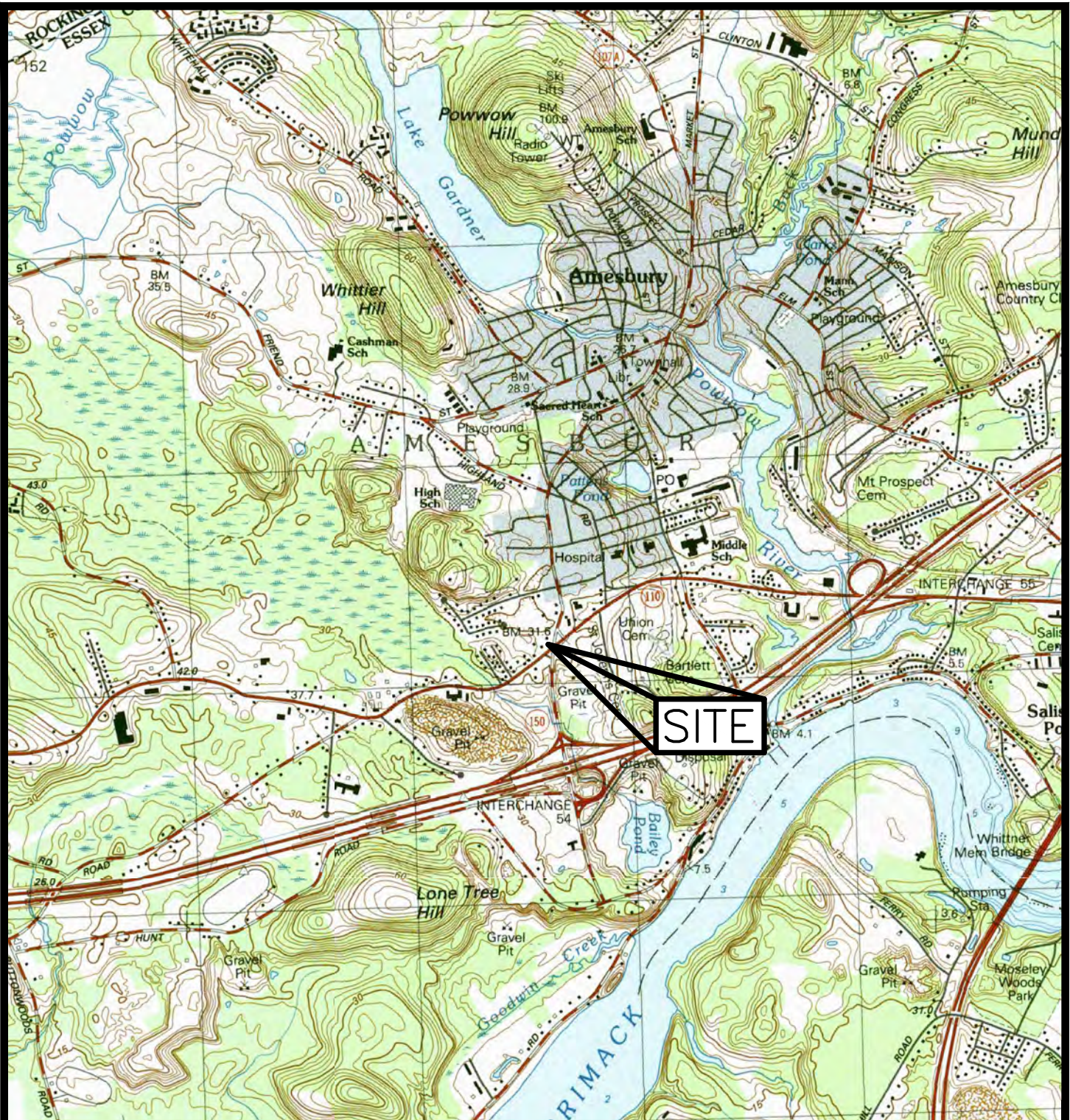
The stormwater facilities were design to attenuate peak flows generated by all storm events up to and including the 100-year storm event. An infiltration rate of 2.41 in/hr was used based on the Rawls Rate of saturated hydraulic conductivity for a loamy sand soil type. Refer to Appendix B for the Stage Storage Curves and TR-20 computer results for the storage characteristics of the subsurface infiltration facilities. Refer to the Site Plans (attached) for design details.

**Erosion and Siltation Control**

Straw wattles and silt fence will be placed at the downhill limit of work prior to the commencement of any construction activity. The integrity of the erosion control devices will be maintained by periodic inspection and replacement as necessary. The straw wattles and silt fence will remain in place until the first course of pavement has been placed and all side slopes have been loamed and seeded and vegetation has been established.







PREPARED BY:

**Engineering Alliance, Inc.**  
 Civil Engineering & Land Planning Consultants  
 194 Central Street  
 Saugus, MA 01906  
 Tel: (781) 231-1349  
 Fax: (781) 417-0020

PROJECT:

**Plan of Land**

41 Hillside Avenue  
 (Tax Map 76 Lot 61)  
 Amesbury, MA 01913

PROJECT: 21-76801

DATE: April 12, 2021

SCALE: 1:25,000

DWG FILE NAME: Figures.dwg

DESIGNED BY: Max Friedman

CHECKED BY: Richard A. Salvo, P.E.

DRAWING TITLE:

**FIGURE 1 - USGS LOCUS MAP**

Page #:

**1 of 5**







PREPARED BY:



**Engineering Alliance, Inc.**  
 Civil Engineering & Land Planning Consultants  
 194 Central Street 1950 Lafayette Road  
 Saugus, MA 01906 Portsmouth, NH 03801  
 Tel: (781) 231-1349 Tel: (603) 610-7100  
 Fax: (781) 417-0020 Fax: (603) 610-7101

PROJECT:

**Plan of Land**

41 Hillside Avenue  
 (Tax Map 76 Lot 61)  
 Amesbury, MA 01913

PROJECT: 21-76801

DATE: April 12, 2021

SCALE: 1"=100'

DWG FILE NAME: Figures.dwg

DESIGNED BY: Max Friedman

CHECKED BY: Richard A. Salvo, P.E.

DRAWING TITLE:

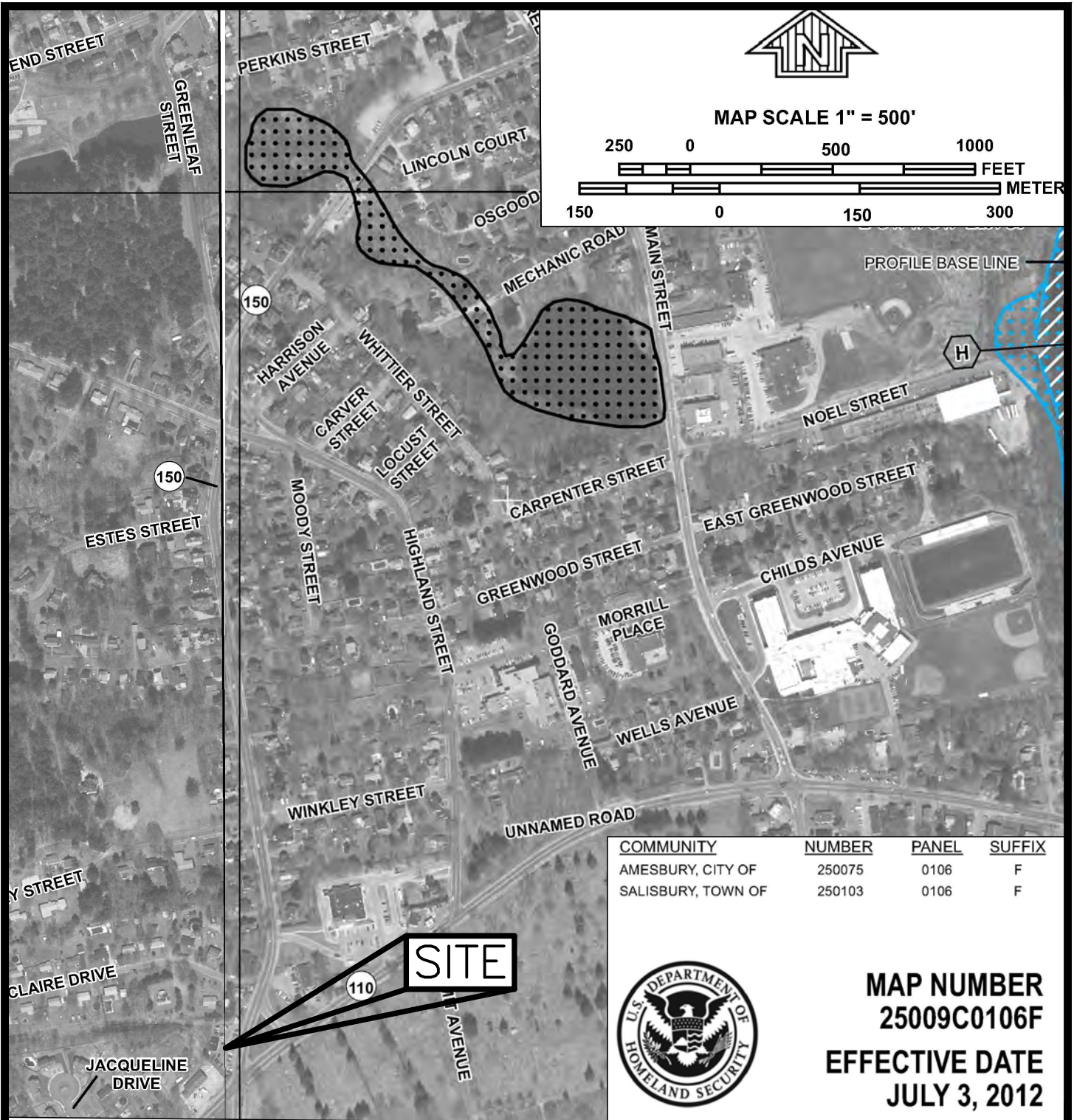
**FIGURE 2 - ORTHO PHOTO**

Page #:

**2 of 5**







PREPARED BY:



**Engineering Alliance, Inc.**  
 Civil Engineering & Land Planning Consultants  
 194 Central Street 1950 Lafayette Road  
 Saugus, MA 01906 Portsmouth, NH 03801  
 Tel: (781) 231-1349 Tel: (603) 610-7100  
 Fax: (781) 417-0020 Fax: (603) 610-7101

PROJECT:

**Plan of Land**

41 Hillside Avenue  
 (Tax Map 76 Lot 61)  
 Amesbury, MA 01913

PROJECT: 21-76801

DATE: April 12, 2021

SCALE: 1"=500'

DWG FILE NAME: Figures.dwg

DESIGNED BY: Max Friedman

CHECKED BY: Richard A. Salvo, P.E.

DRAWING TITLE:

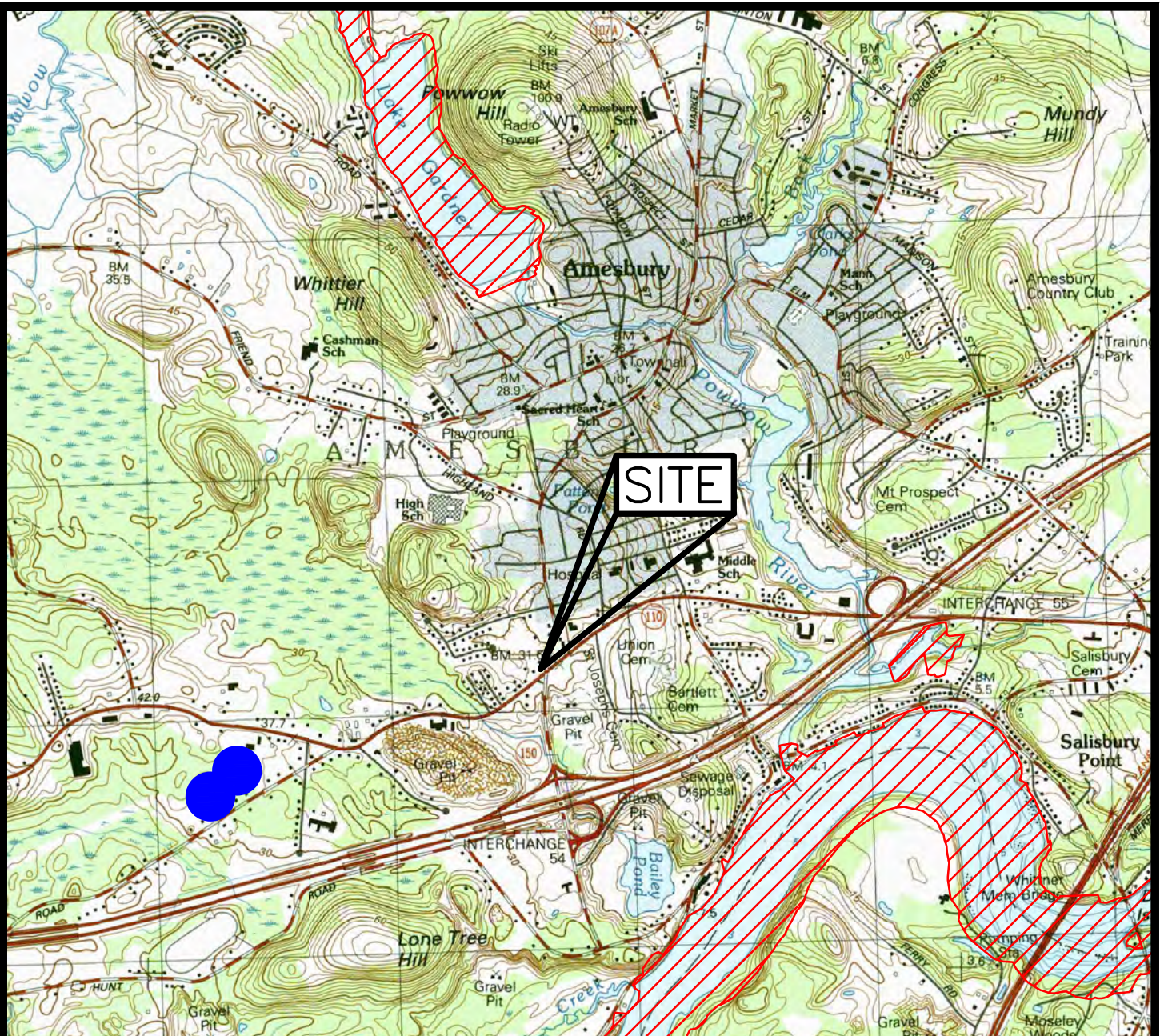
**FIGURE 3 - FEMA FLOOD MAP**

Page #:




**3 of 5**







**LEGEND:**

-  = NHESP CERTIFIED VERNAL POOL
-  = NHESP ESTIMATED HABITATS OF RARE SPECIES
-  = NHESP PRIORITY HABITATS OF RARE SPECIES

PREPARED BY:



**Engineering Alliance, Inc.**  
 Civil Engineering & Land Planning Consultants  
 194 Central Street  
 Saugus, MA 01906  
 Tel: (781) 231-1349  
 Fax: (781) 417-0020

1950 Lafayette Road  
 Portsmouth, NH 03801  
 Tel: (603) 610-7100  
 Fax: (603) 610-7101

PROJECT:

## Plan of Land

41 Hillside Avenue  
 (Tax Map 76 Lot 61)  
 Amesbury, MA 01913

PROJECT: 21-76801

SCALE: 1:25,000

DESIGNED BY: Max Friedman

DATE: April 12, 2021

DWG FILE NAME: Figures.dwg

CHECKED BY: Richard A. Salvo, P.E.

DRAWING TITLE:

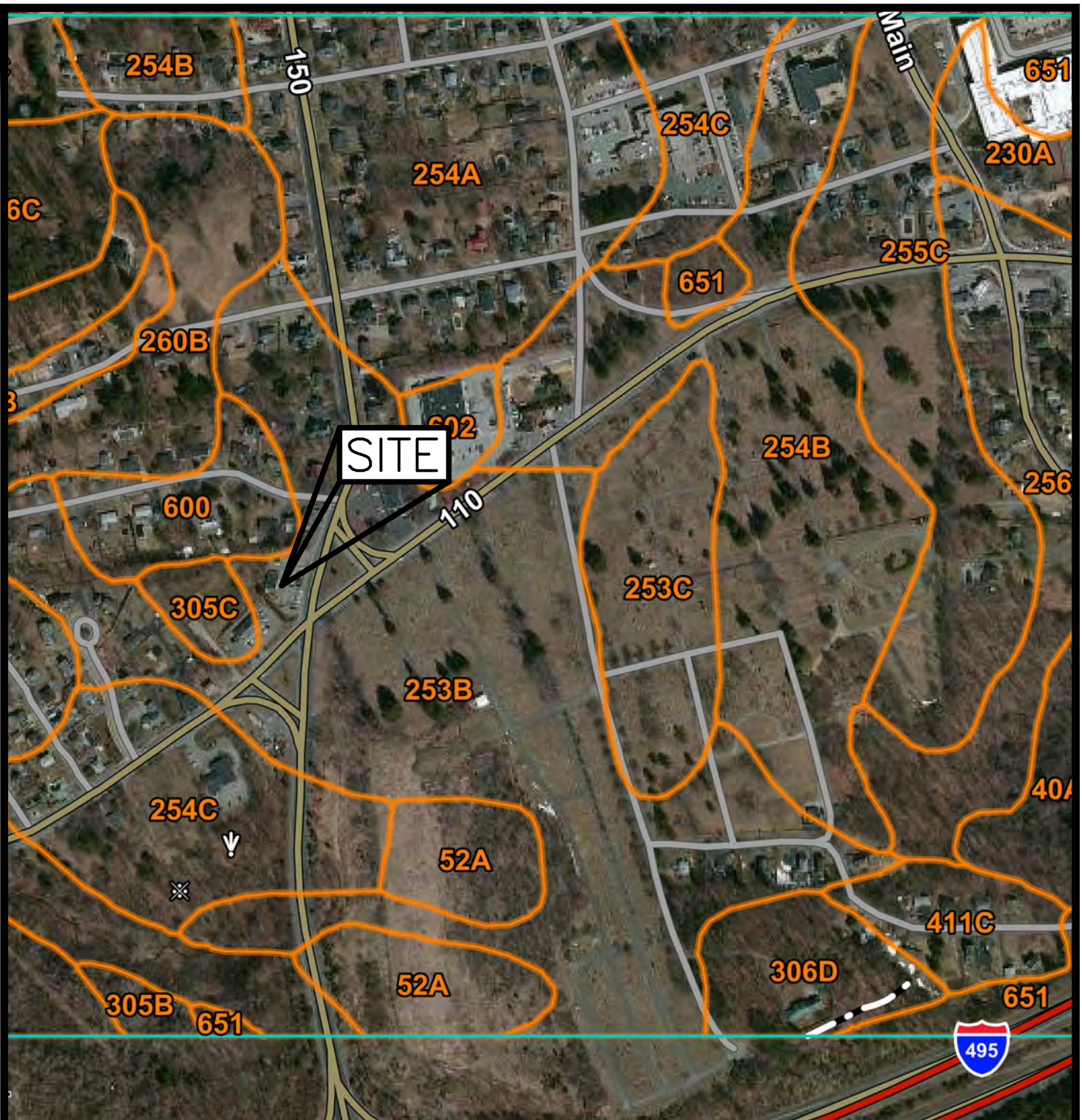
# FIGURE 4 - NATURAL HERITAGE MAP

Page #:

4 of 5







PREPARED BY:



**Engineering Alliance, Inc.**  
 Civil Engineering & Land Planning Consultants  
 194 Central Street  
 Saugus, MA 01906  
 Tel: (781) 231-1349  
 Fax: (781) 417-0020  
 1950 Lafayette Road  
 Portsmouth, NH 03801  
 Tel: (603) 610-7100  
 Fax: (603) 610-7101

PROJECT:

## Plan of Land

41 Hillside Avenue  
 (Tax Map 76 Lot 61)  
 Amesbury, MA 01913

PROJECT: 21-76801

DATE: April 12, 2021

SCALE: 1"=400'

DWG FILE NAME: Figures.dwg

DESIGNED BY: Max Friedman

CHECKED BY: Richard A. Salvo, P.E.

DRAWING TITLE:

# FIGURE 5 - SOILS MAP

Page #:

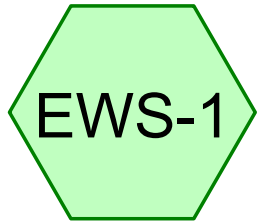
5 of 5



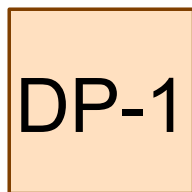
## **APPENDIX A**

**Existing Conditions Drainage Calculations  
Existing Watershed Plan**

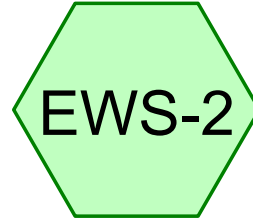




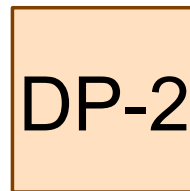
EWS-1



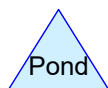
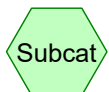
Hillside Ave



EWS-2



Allenclair Drive



**Routing Diagram for Existing Conditions**

Prepared by Engineering Alliance Inc., Printed 10/27/2021  
HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

## Existing Conditions

Prepared by Engineering Alliance Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Printed 10/27/2021

Page 2

### Rainfall Events Listing

Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
1	2-year	Type III 24-hr		Default	24.00	1	3.38	2
2	10-year	Type III 24-hr		Default	24.00	1	5.35	2
3	25-year	Type III 24-hr		Default	24.00	1	6.58	2
4	100-year	Type III 24-hr		Default	24.00	1	8.47	2

## Existing Conditions

Prepared by Engineering Alliance Inc.

Printed 10/27/2021

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Page 3

### Ground Covers (all nodes)

HSG-A (sq-ft)	HSG-B (sq-ft)	HSG-C (sq-ft)	HSG-D (sq-ft)	Other (sq-ft)	Total (sq-ft)	Ground Cover
3,853	0	0	0	0	3,853	50-75% Grass cover, Fair
0	0	4,956	0	0	4,956	Paved parking
13,395	0	0	0	0	13,395	Roofs
1,456	0	0	0	0	1,456	Woods/grass comb., Poor
<b>18,704</b>	<b>0</b>	<b>4,956</b>	<b>0</b>	<b>0</b>	<b>23,660</b>	<b>TOTAL AREA</b>

## Existing Conditions

Prepared by Engineering Alliance Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 2-year Rainfall=3.38"

Printed 10/27/2021

Page 4

Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN

Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

### Subcatchment EWS-1: EWS-1

Runoff Area=18,177 sf 76.40% Impervious Runoff Depth>2.07"

Tc=5.0 min CN=87 Runoff=1.01 cfs 3,141 cf

### Subcatchment EWS-2: EWS-2

Runoff Area=5,483 sf 81.42% Impervious Runoff Depth>2.25"

Tc=5.0 min CN=89 Runoff=0.33 cfs 1,026 cf

### Reach DP-1: Hillside Ave

Inflow=1.01 cfs 3,141 cf

Outflow=1.01 cfs 3,141 cf

### Reach DP-2: Allenclair Drive

Inflow=0.33 cfs 1,026 cf

Outflow=0.33 cfs 1,026 cf

**Total Runoff Area = 23,660 sf Runoff Volume = 4,167 cf Average Runoff Depth = 2.11"**  
**22.44% Pervious = 5,309 sf 77.56% Impervious = 18,351 sf**



## Existing Conditions

Prepared by Engineering Alliance Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 2-year Rainfall=3.38"

Printed 10/27/2021

Page 5

### Summary for Subcatchment EWS-1: EWS-1

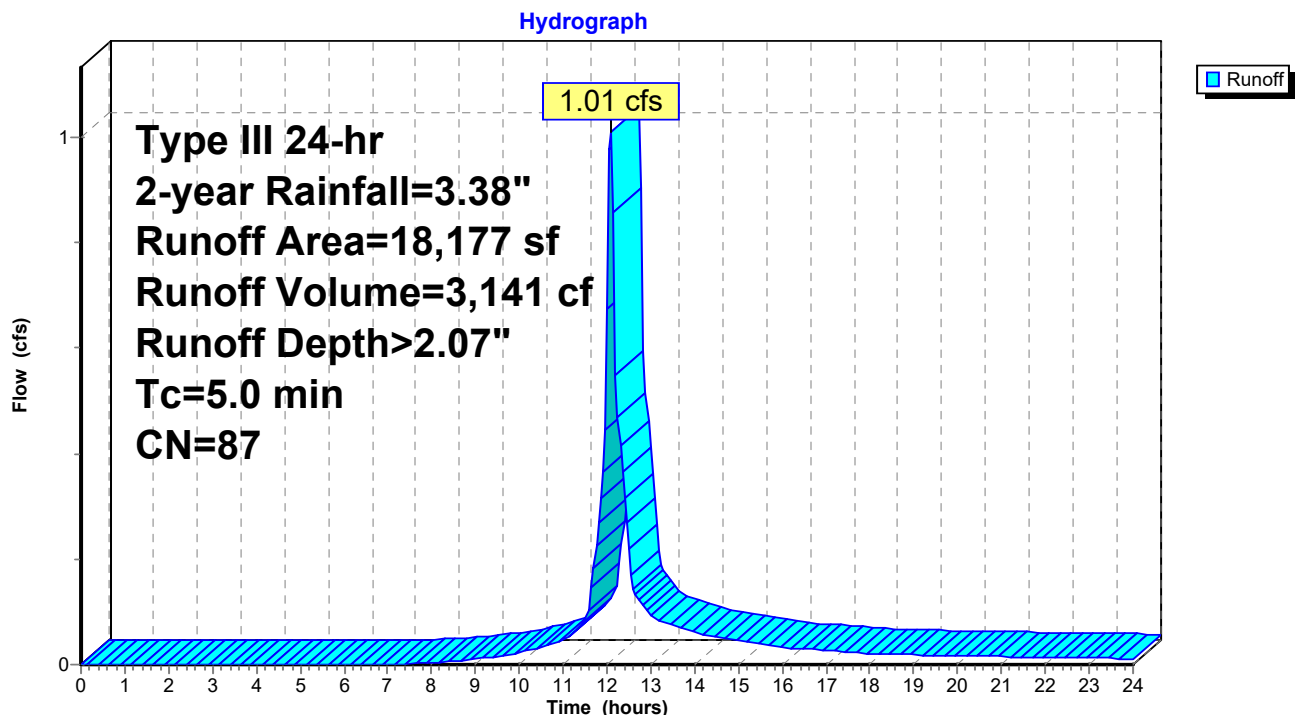
Runoff = 1.01 cfs @ 12.08 hrs, Volume= 3,141 cf, Depth> 2.07"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
Type III 24-hr 2-year Rainfall=3.38"

Area (sf)	CN	Description
1,296	98	Paved parking, HSG C
12,591	98	Roofs, HSG A
2,834	49	50-75% Grass cover, Fair, HSG A
1,456	57	Woods/grass comb., Poor, HSG A
18,177	87	Weighted Average
4,290		23.60% Pervious Area
13,887		76.40% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

### Subcatchment EWS-1: EWS-1



## Existing Conditions

Prepared by Engineering Alliance Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 2-year Rainfall=3.38"

Printed 10/27/2021

Page 6

### Summary for Subcatchment EWS-2: EWS-2

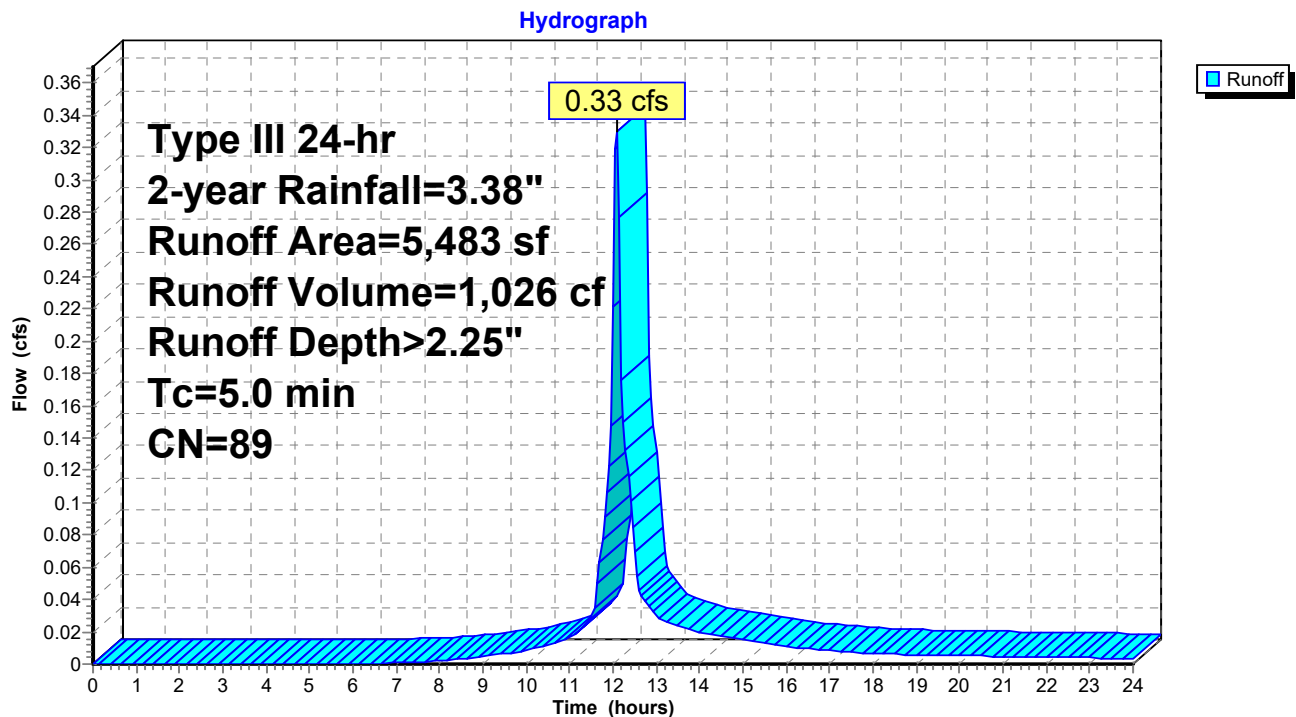
Runoff = 0.33 cfs @ 12.07 hrs, Volume= 1,026 cf, Depth> 2.25"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
Type III 24-hr 2-year Rainfall=3.38"

Area (sf)	CN	Description
804	98	Roofs, HSG A
3,660	98	Paved parking, HSG C
1,019	49	50-75% Grass cover, Fair, HSG A
5,483	89	Weighted Average
1,019		18.58% Pervious Area
4,464		81.42% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

### Subcatchment EWS-2: EWS-2



## Existing Conditions

Prepared by Engineering Alliance Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 2-year Rainfall=3.38"

Printed 10/27/2021

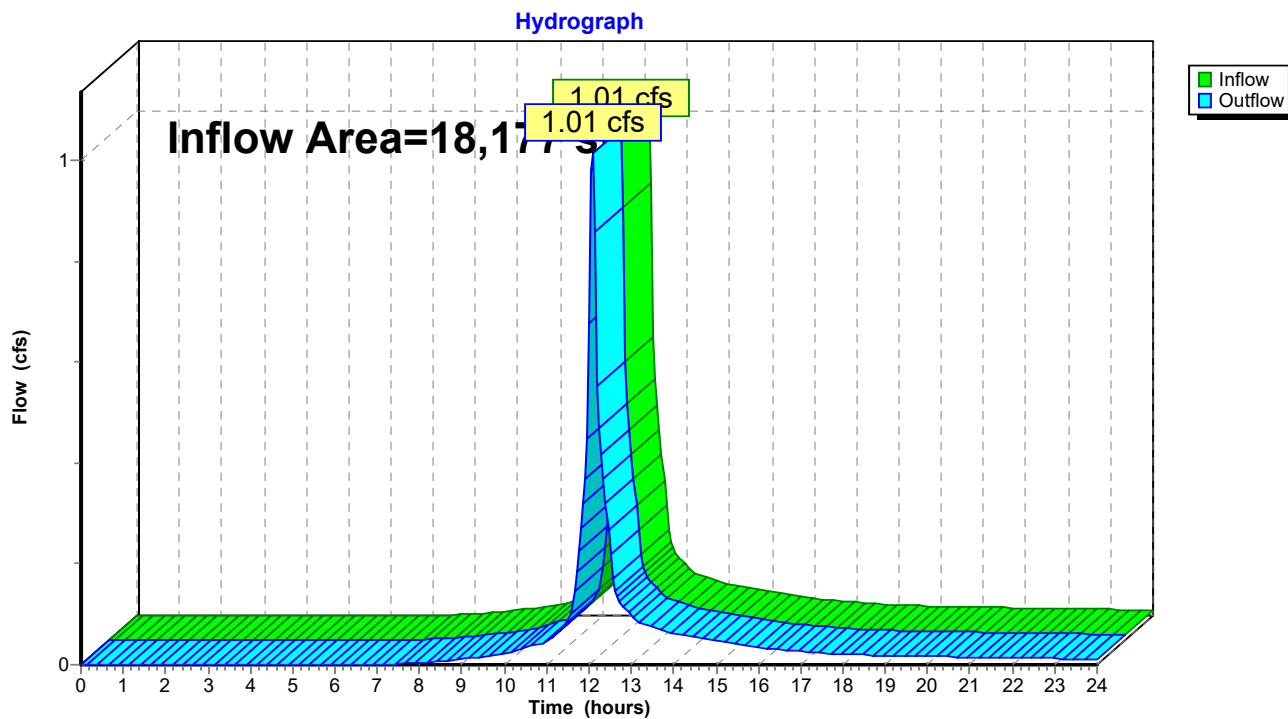
Page 7

### Summary for Reach DP-1: Hillside Ave

Inflow Area = 18,177 sf, 76.40% Impervious, Inflow Depth > 2.07" for 2-year event  
Inflow = 1.01 cfs @ 12.08 hrs, Volume= 3,141 cf  
Outflow = 1.01 cfs @ 12.08 hrs, Volume= 3,141 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

### Reach DP-1: Hillside Ave



## Existing Conditions

Prepared by Engineering Alliance Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 2-year Rainfall=3.38"

Printed 10/27/2021

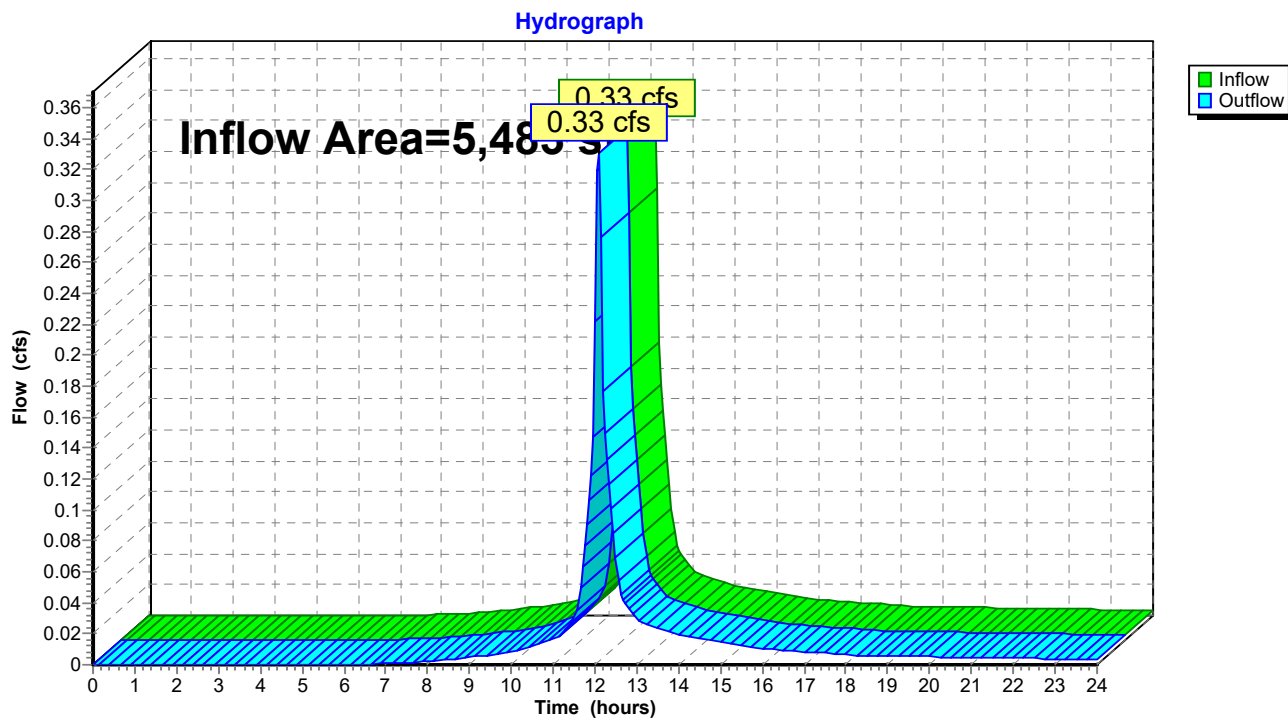
Page 8

### Summary for Reach DP-2: Allenclair Drive

Inflow Area = 5,483 sf, 81.42% Impervious, Inflow Depth > 2.25" for 2-year event  
Inflow = 0.33 cfs @ 12.07 hrs, Volume= 1,026 cf  
Outflow = 0.33 cfs @ 12.07 hrs, Volume= 1,026 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

### Reach DP-2: Allenclair Drive



## Existing Conditions

Prepared by Engineering Alliance Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 10-year Rainfall=5.35"

Printed 10/27/2021

Page 9

Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN

Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

### Subcatchment EWS-1: EWS-1

Runoff Area=18,177 sf 76.40% Impervious Runoff Depth>3.90"

Tc=5.0 min CN=87 Runoff=1.88 cfs 5,901 cf

### Subcatchment EWS-2: EWS-2

Runoff Area=5,483 sf 81.42% Impervious Runoff Depth>4.11"

Tc=5.0 min CN=89 Runoff=0.59 cfs 1,876 cf

### Reach DP-1: Hillside Ave

Inflow=1.88 cfs 5,901 cf

Outflow=1.88 cfs 5,901 cf

### Reach DP-2: Allenclair Drive

Inflow=0.59 cfs 1,876 cf

Outflow=0.59 cfs 1,876 cf

**Total Runoff Area = 23,660 sf Runoff Volume = 7,777 cf Average Runoff Depth = 3.94"**  
**22.44% Pervious = 5,309 sf 77.56% Impervious = 18,351 sf**

## Existing Conditions

Prepared by Engineering Alliance Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 10-year Rainfall=5.35"

Printed 10/27/2021

Page 10

### Summary for Subcatchment EWS-1: EWS-1

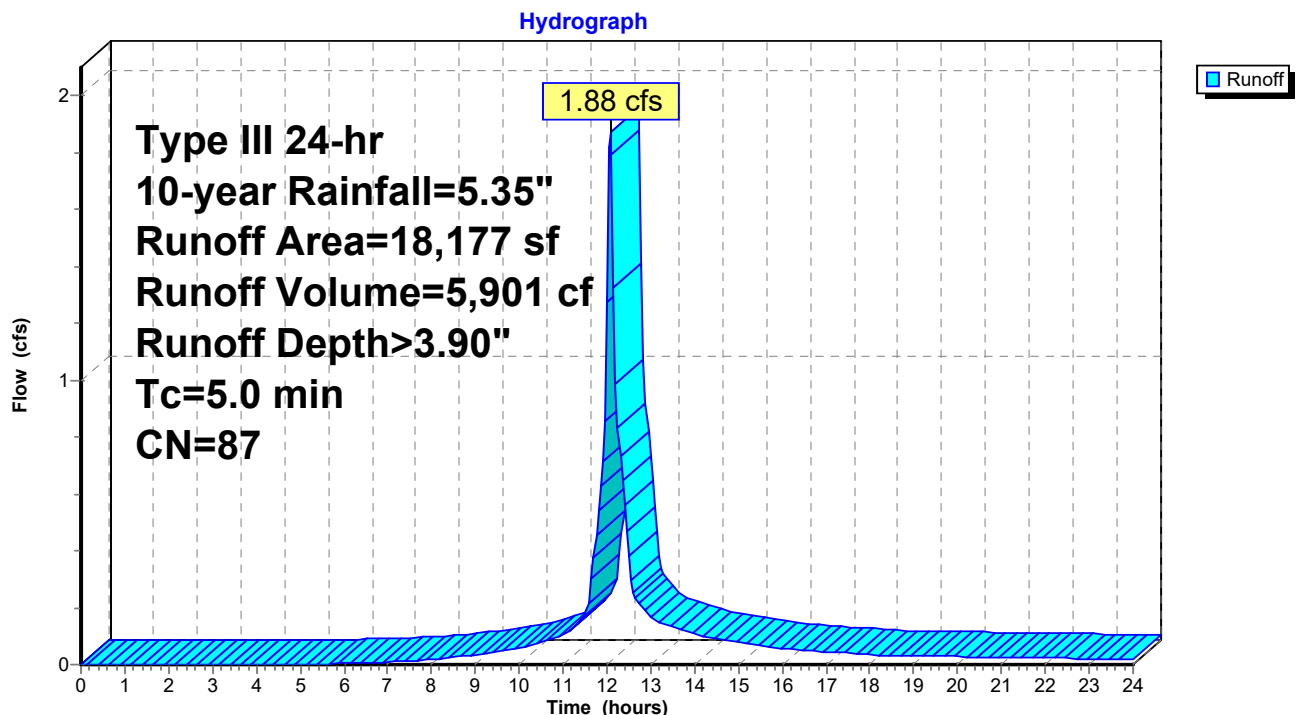
Runoff = 1.88 cfs @ 12.07 hrs, Volume= 5,901 cf, Depth> 3.90"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
Type III 24-hr 10-year Rainfall=5.35"

Area (sf)	CN	Description
1,296	98	Paved parking, HSG C
12,591	98	Roofs, HSG A
2,834	49	50-75% Grass cover, Fair, HSG A
1,456	57	Woods/grass comb., Poor, HSG A
18,177	87	Weighted Average
4,290		23.60% Pervious Area
13,887		76.40% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

### Subcatchment EWS-1: EWS-1



## Existing Conditions

Prepared by Engineering Alliance Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 10-year Rainfall=5.35"

Printed 10/27/2021

Page 11

### Summary for Subcatchment EWS-2: EWS-2

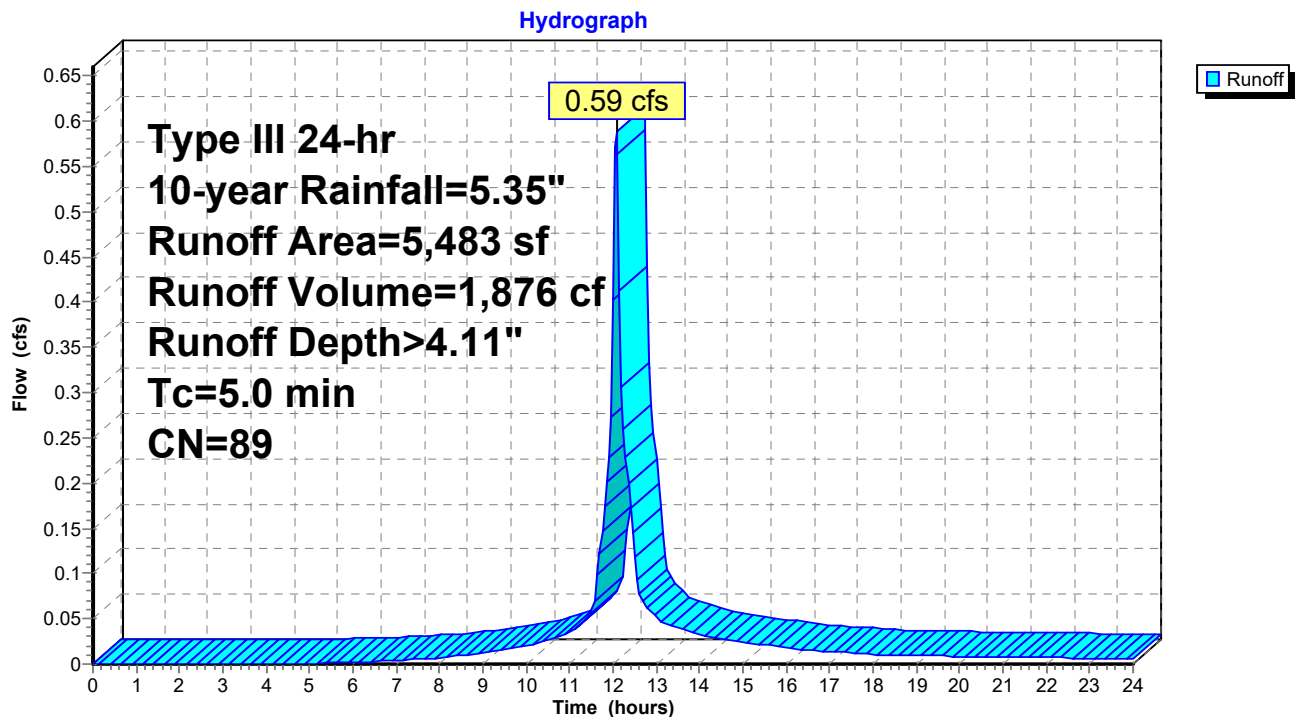
Runoff = 0.59 cfs @ 12.07 hrs, Volume= 1,876 cf, Depth> 4.11"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
Type III 24-hr 10-year Rainfall=5.35"

Area (sf)	CN	Description
804	98	Roofs, HSG A
3,660	98	Paved parking, HSG C
1,019	49	50-75% Grass cover, Fair, HSG A
5,483	89	Weighted Average
1,019		18.58% Pervious Area
4,464		81.42% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

### Subcatchment EWS-2: EWS-2



## Existing Conditions

Prepared by Engineering Alliance Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 10-year Rainfall=5.35"

Printed 10/27/2021

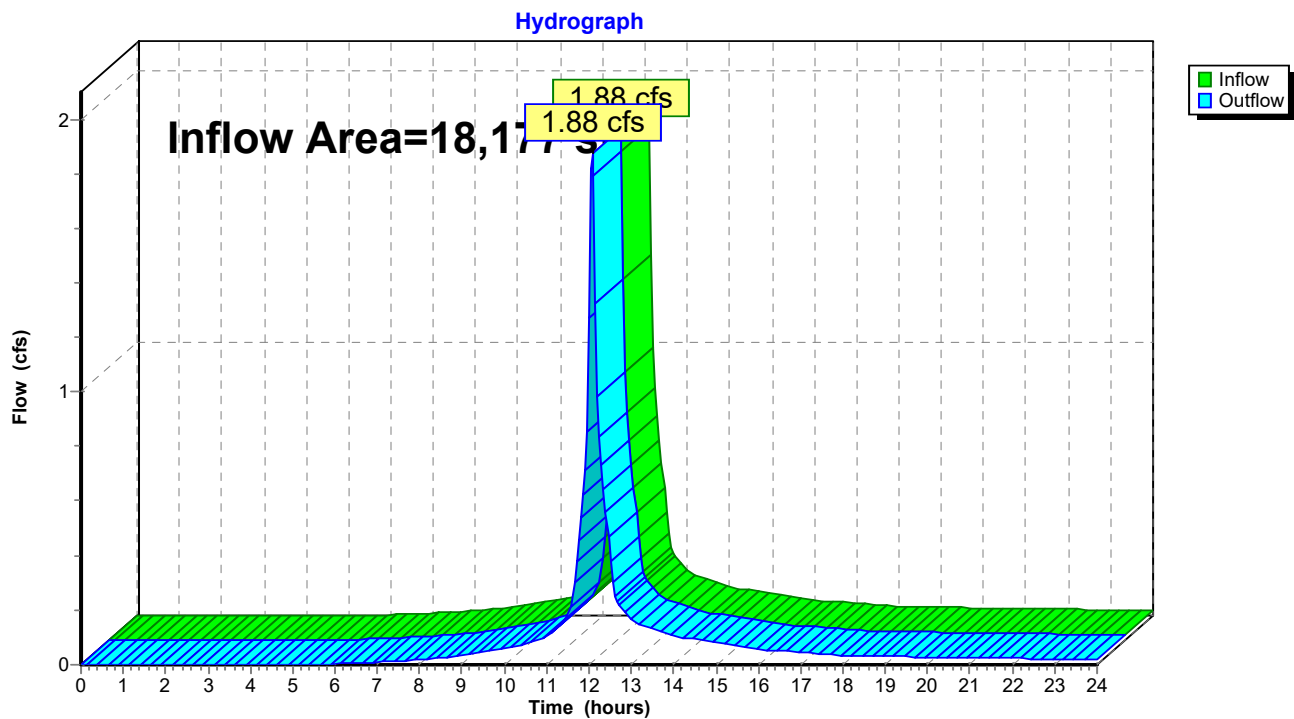
Page 12

### Summary for Reach DP-1: Hillside Ave

Inflow Area = 18,177 sf, 76.40% Impervious, Inflow Depth > 3.90" for 10-year event  
Inflow = 1.88 cfs @ 12.07 hrs, Volume= 5,901 cf  
Outflow = 1.88 cfs @ 12.07 hrs, Volume= 5,901 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

### Reach DP-1: Hillside Ave





## Existing Conditions

Prepared by Engineering Alliance Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 10-year Rainfall=5.35"

Printed 10/27/2021

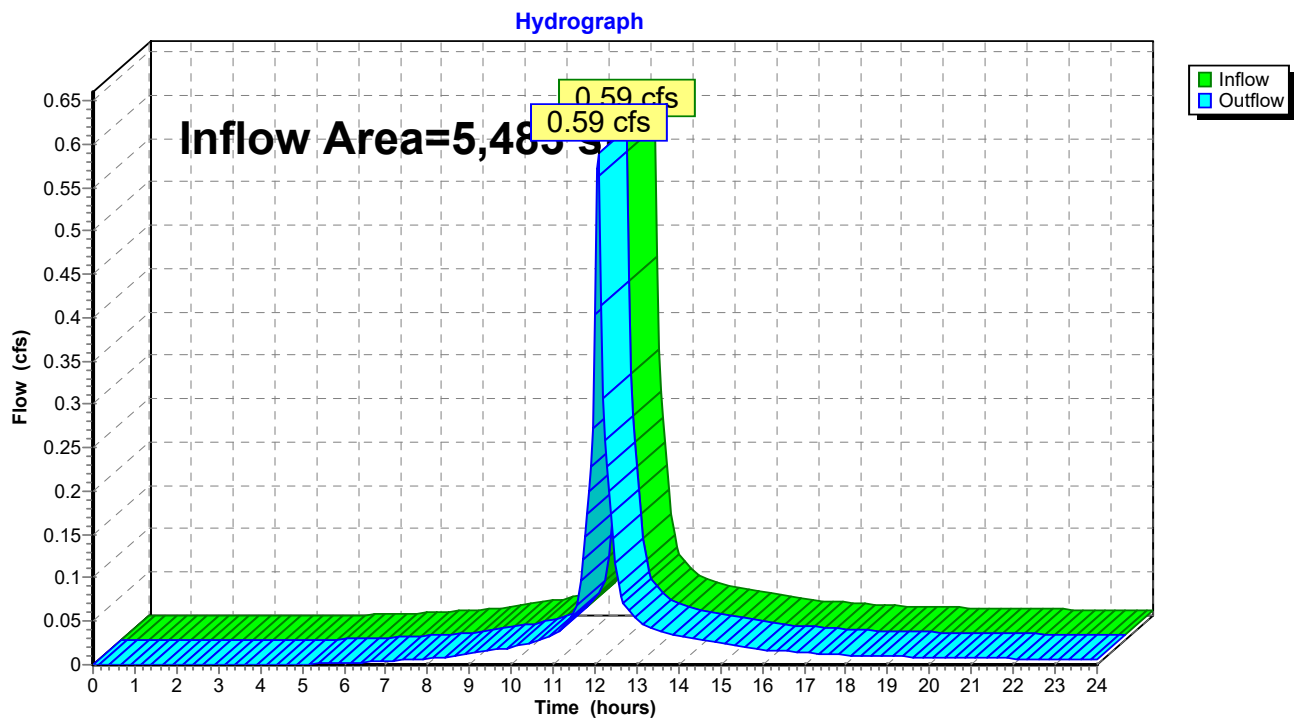
Page 13

### Summary for Reach DP-2: Allenclair Drive

Inflow Area = 5,483 sf, 81.42% Impervious, Inflow Depth > 4.11" for 10-year event  
Inflow = 0.59 cfs @ 12.07 hrs, Volume= 1,876 cf  
Outflow = 0.59 cfs @ 12.07 hrs, Volume= 1,876 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

### Reach DP-2: Allenclair Drive



## Existing Conditions

Prepared by Engineering Alliance Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 25-year Rainfall=6.58"

Printed 10/27/2021

Page 14

Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN

Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

### Subcatchment EWS-1: EWS-1

Runoff Area=18,177 sf 76.40% Impervious Runoff Depth>5.07"

Tc=5.0 min CN=87 Runoff=2.41 cfs 7,682 cf

### Subcatchment EWS-2: EWS-2

Runoff Area=5,483 sf 81.42% Impervious Runoff Depth>5.30"

Tc=5.0 min CN=89 Runoff=0.75 cfs 2,420 cf

### Reach DP-1: Hillside Ave

Inflow=2.41 cfs 7,682 cf

Outflow=2.41 cfs 7,682 cf

### Reach DP-2: Allenclair Drive

Inflow=0.75 cfs 2,420 cf

Outflow=0.75 cfs 2,420 cf

**Total Runoff Area = 23,660 sf Runoff Volume = 10,101 cf Average Runoff Depth = 5.12"**  
**22.44% Pervious = 5,309 sf 77.56% Impervious = 18,351 sf**

## Existing Conditions

Prepared by Engineering Alliance Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 25-year Rainfall=6.58"

Printed 10/27/2021

Page 15

### Summary for Subcatchment EWS-1: EWS-1

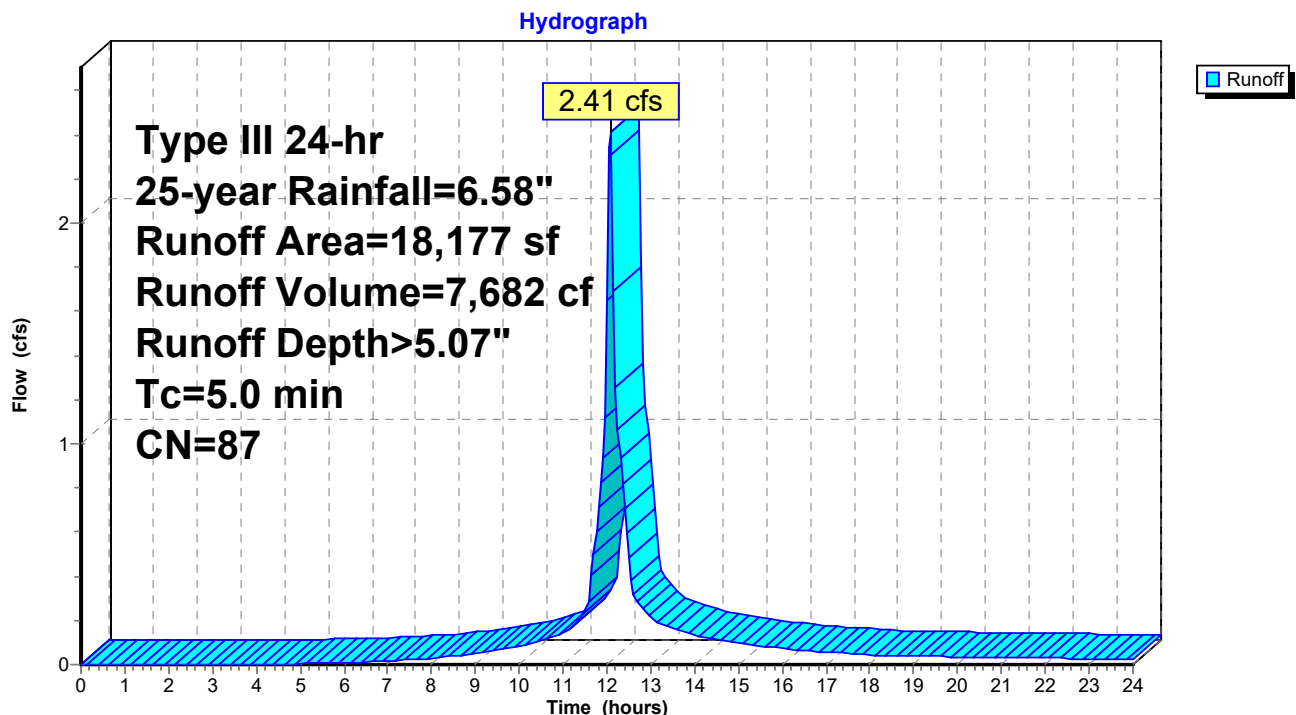
Runoff = 2.41 cfs @ 12.07 hrs, Volume= 7,682 cf, Depth> 5.07"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
Type III 24-hr 25-year Rainfall=6.58"

Area (sf)	CN	Description
1,296	98	Paved parking, HSG C
12,591	98	Roofs, HSG A
2,834	49	50-75% Grass cover, Fair, HSG A
1,456	57	Woods/grass comb., Poor, HSG A
18,177	87	Weighted Average
4,290		23.60% Pervious Area
13,887		76.40% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

### Subcatchment EWS-1: EWS-1



## Existing Conditions

Prepared by Engineering Alliance Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 25-year Rainfall=6.58"

Printed 10/27/2021

Page 16

### Summary for Subcatchment EWS-2: EWS-2

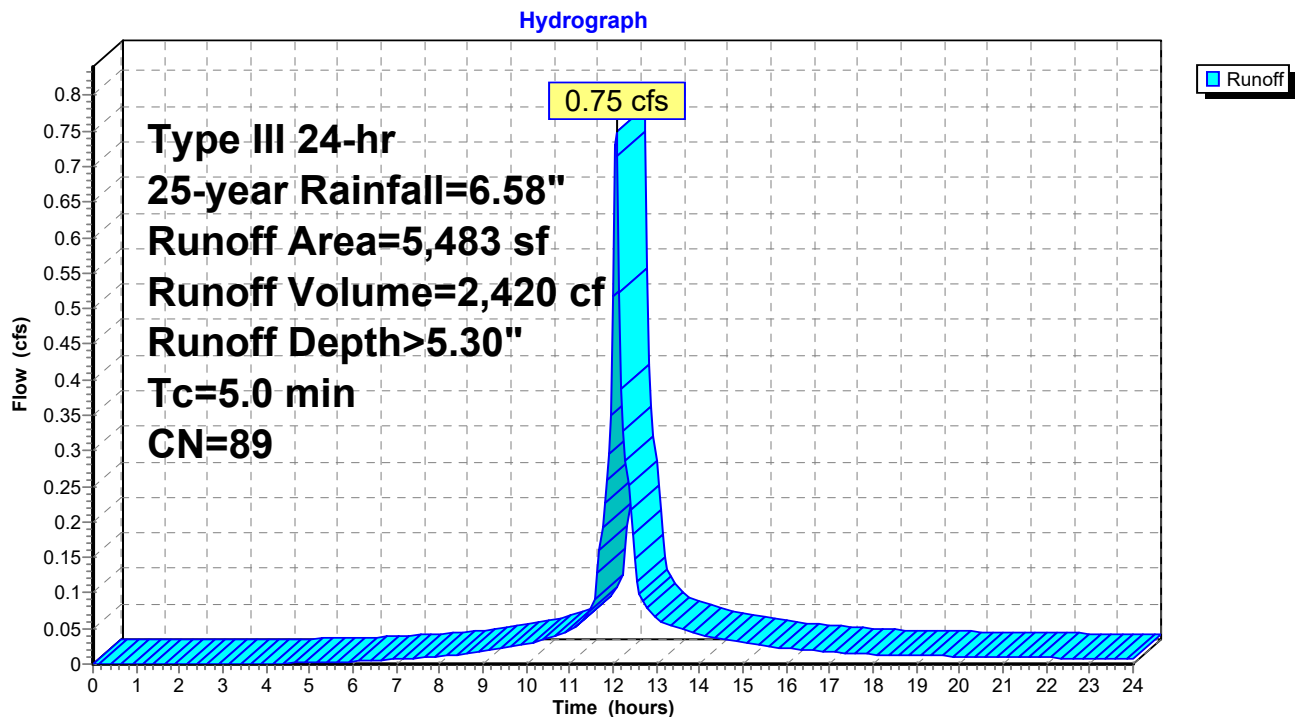
Runoff = 0.75 cfs @ 12.07 hrs, Volume= 2,420 cf, Depth> 5.30"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
Type III 24-hr 25-year Rainfall=6.58"

Area (sf)	CN	Description
804	98	Roofs, HSG A
3,660	98	Paved parking, HSG C
1,019	49	50-75% Grass cover, Fair, HSG A
5,483	89	Weighted Average
1,019		18.58% Pervious Area
4,464		81.42% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

### Subcatchment EWS-2: EWS-2



## Existing Conditions

Prepared by Engineering Alliance Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 25-year Rainfall=6.58"

Printed 10/27/2021

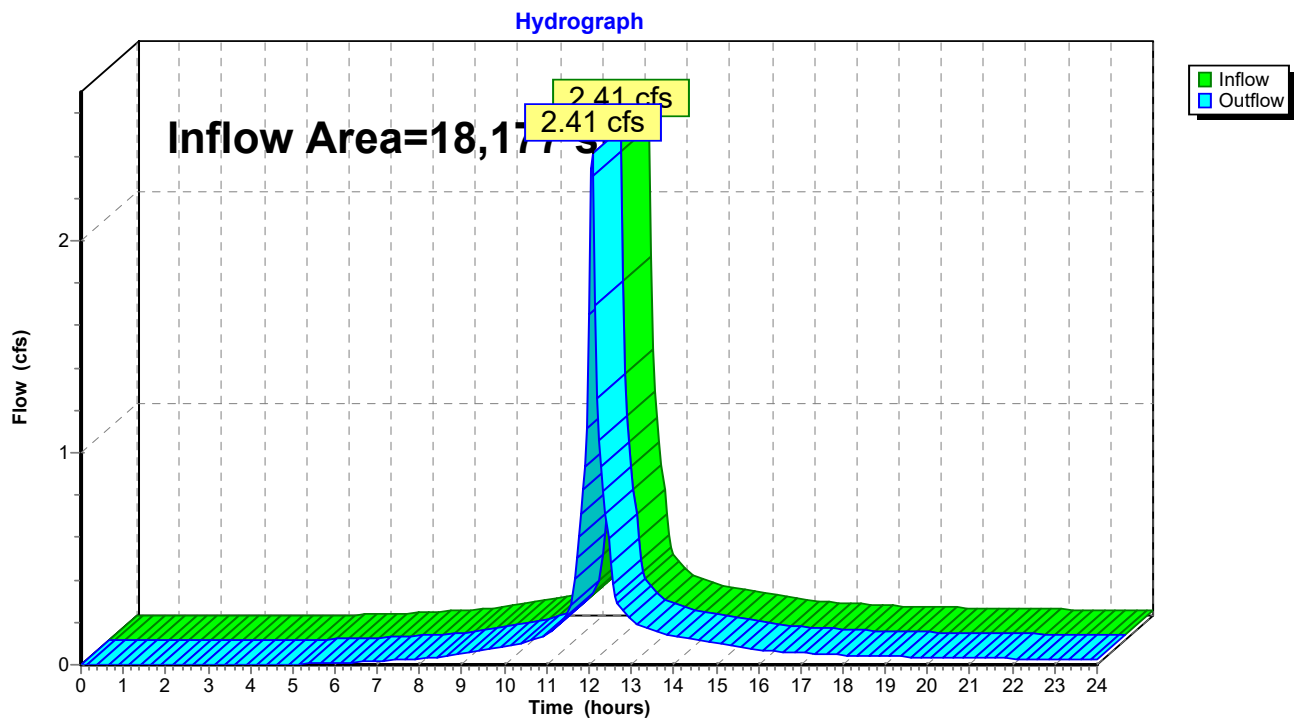
Page 17

### Summary for Reach DP-1: Hillside Ave

Inflow Area = 18,177 sf, 76.40% Impervious, Inflow Depth > 5.07" for 25-year event  
Inflow = 2.41 cfs @ 12.07 hrs, Volume= 7,682 cf  
Outflow = 2.41 cfs @ 12.07 hrs, Volume= 7,682 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

### Reach DP-1: Hillside Ave



## Existing Conditions

Prepared by Engineering Alliance Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 25-year Rainfall=6.58"

Printed 10/27/2021

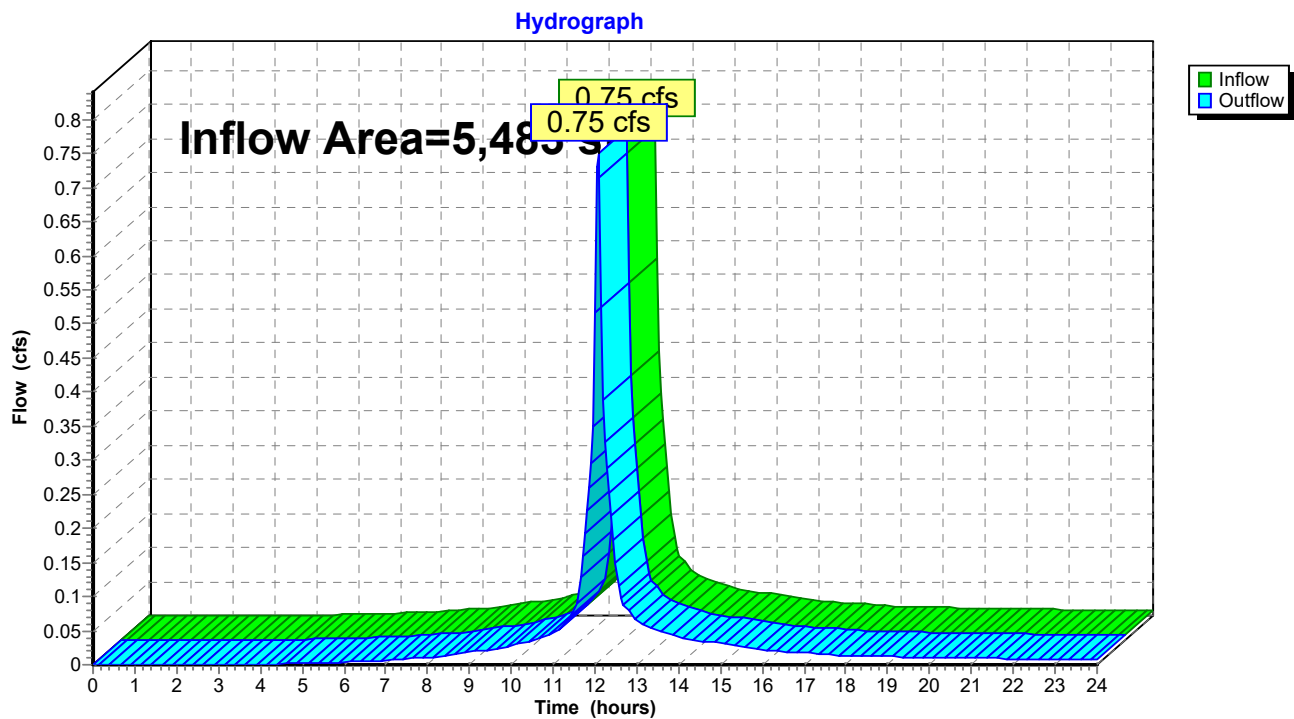
Page 18

### Summary for Reach DP-2: Allenclair Drive

Inflow Area = 5,483 sf, 81.42% Impervious, Inflow Depth > 5.30" for 25-year event  
Inflow = 0.75 cfs @ 12.07 hrs, Volume= 2,420 cf  
Outflow = 0.75 cfs @ 12.07 hrs, Volume= 2,420 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

### Reach DP-2: Allenclair Drive



## Existing Conditions

Prepared by Engineering Alliance Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 100-year Rainfall=8.47"

Printed 10/27/2021

Page 19

Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN

Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

### Subcatchment EWS-1: EWS-1

Runoff Area=18,177 sf 76.40% Impervious Runoff Depth>6.90"

Tc=5.0 min CN=87 Runoff=3.23 cfs 10,458 cf

### Subcatchment EWS-2: EWS-2

Runoff Area=5,483 sf 81.42% Impervious Runoff Depth>7.14"

Tc=5.0 min CN=89 Runoff=1.00 cfs 3,264 cf

### Reach DP-1: Hillside Ave

Inflow=3.23 cfs 10,458 cf

Outflow=3.23 cfs 10,458 cf

### Reach DP-2: Allenclair Drive

Inflow=1.00 cfs 3,264 cf

Outflow=1.00 cfs 3,264 cf

**Total Runoff Area = 23,660 sf Runoff Volume = 13,723 cf Average Runoff Depth = 6.96"**  
**22.44% Pervious = 5,309 sf 77.56% Impervious = 18,351 sf**

## Existing Conditions

Prepared by Engineering Alliance Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 100-year Rainfall=8.47"

Printed 10/27/2021

Page 20

### Summary for Subcatchment EWS-1: EWS-1

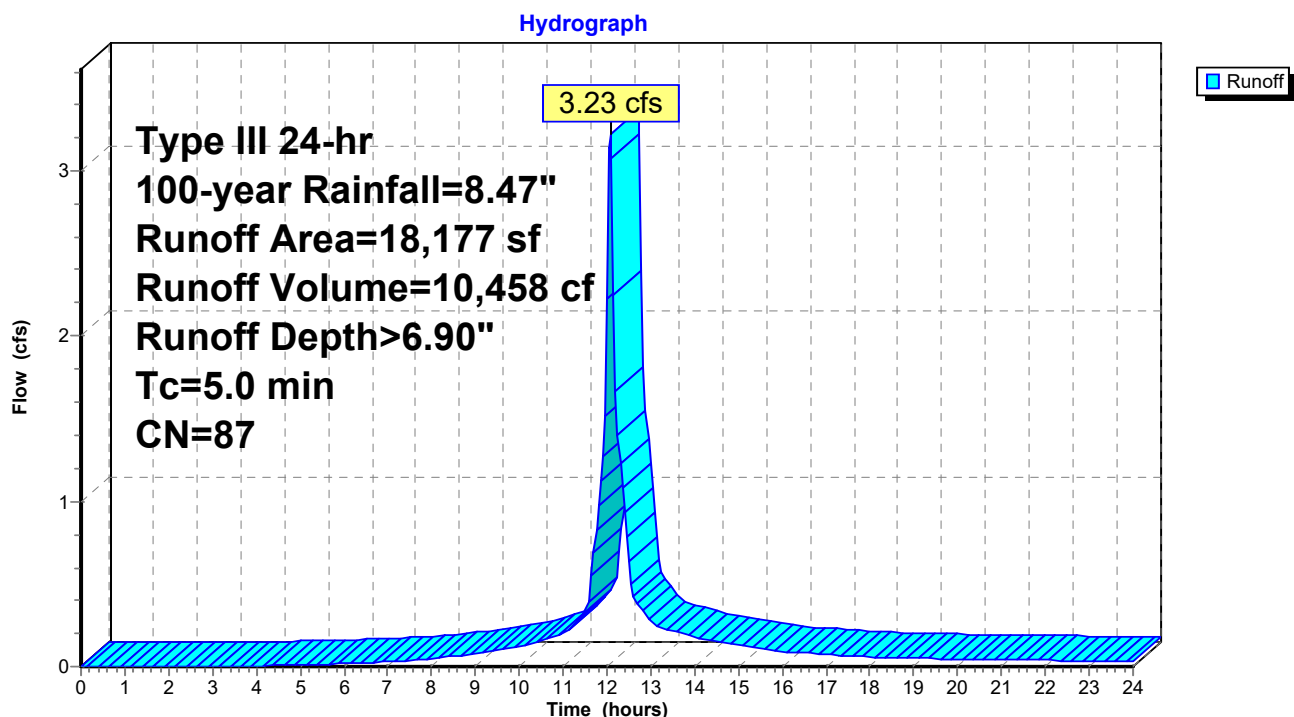
Runoff = 3.23 cfs @ 12.07 hrs, Volume= 10,458 cf, Depth> 6.90"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
Type III 24-hr 100-year Rainfall=8.47"

Area (sf)	CN	Description
1,296	98	Paved parking, HSG C
12,591	98	Roofs, HSG A
2,834	49	50-75% Grass cover, Fair, HSG A
1,456	57	Woods/grass comb., Poor, HSG A
18,177	87	Weighted Average
4,290		23.60% Pervious Area
13,887		76.40% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

### Subcatchment EWS-1: EWS-1





## Existing Conditions

Prepared by Engineering Alliance Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 100-year Rainfall=8.47"

Printed 10/27/2021

Page 21

### Summary for Subcatchment EWS-2: EWS-2

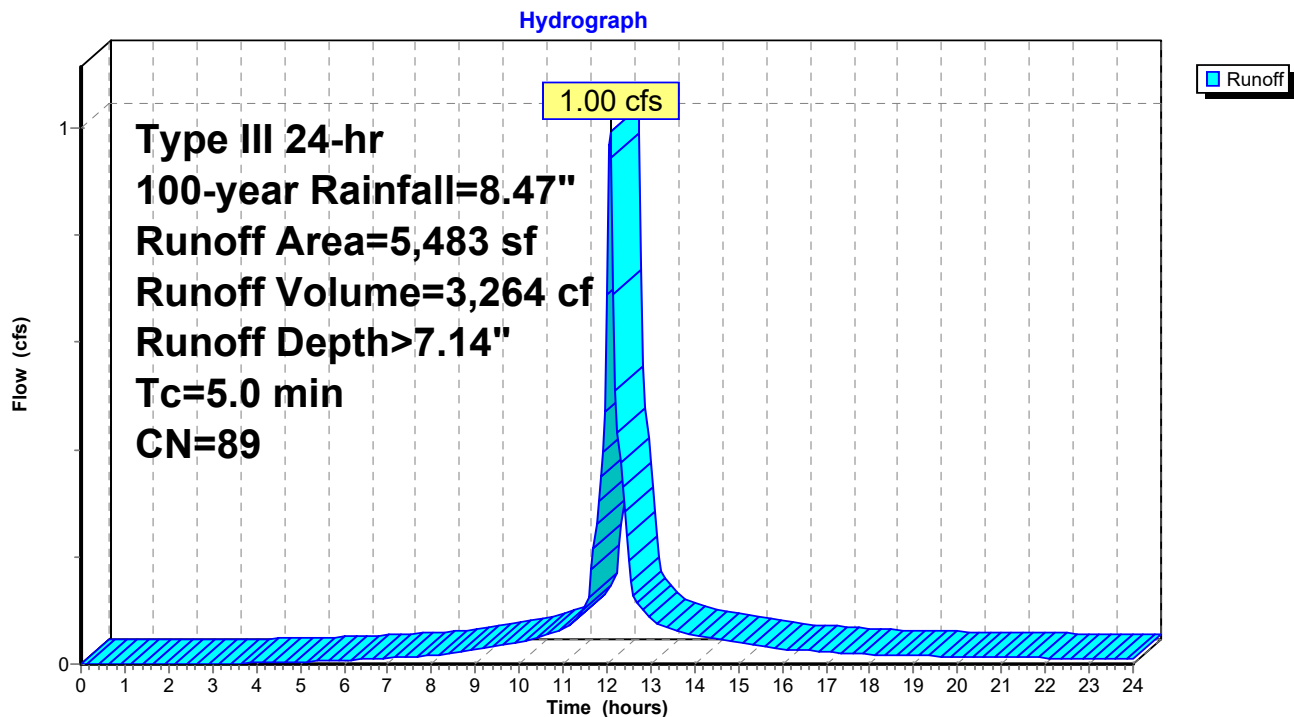
Runoff = 1.00 cfs @ 12.07 hrs, Volume= 3,264 cf, Depth> 7.14"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
Type III 24-hr 100-year Rainfall=8.47"

Area (sf)	CN	Description
804	98	Roofs, HSG A
3,660	98	Paved parking, HSG C
1,019	49	50-75% Grass cover, Fair, HSG A
5,483	89	Weighted Average
1,019		18.58% Pervious Area
4,464		81.42% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

### Subcatchment EWS-2: EWS-2



## Existing Conditions

Prepared by Engineering Alliance Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 100-year Rainfall=8.47"

Printed 10/27/2021

Page 22

### Summary for Reach DP-1: Hillside Ave

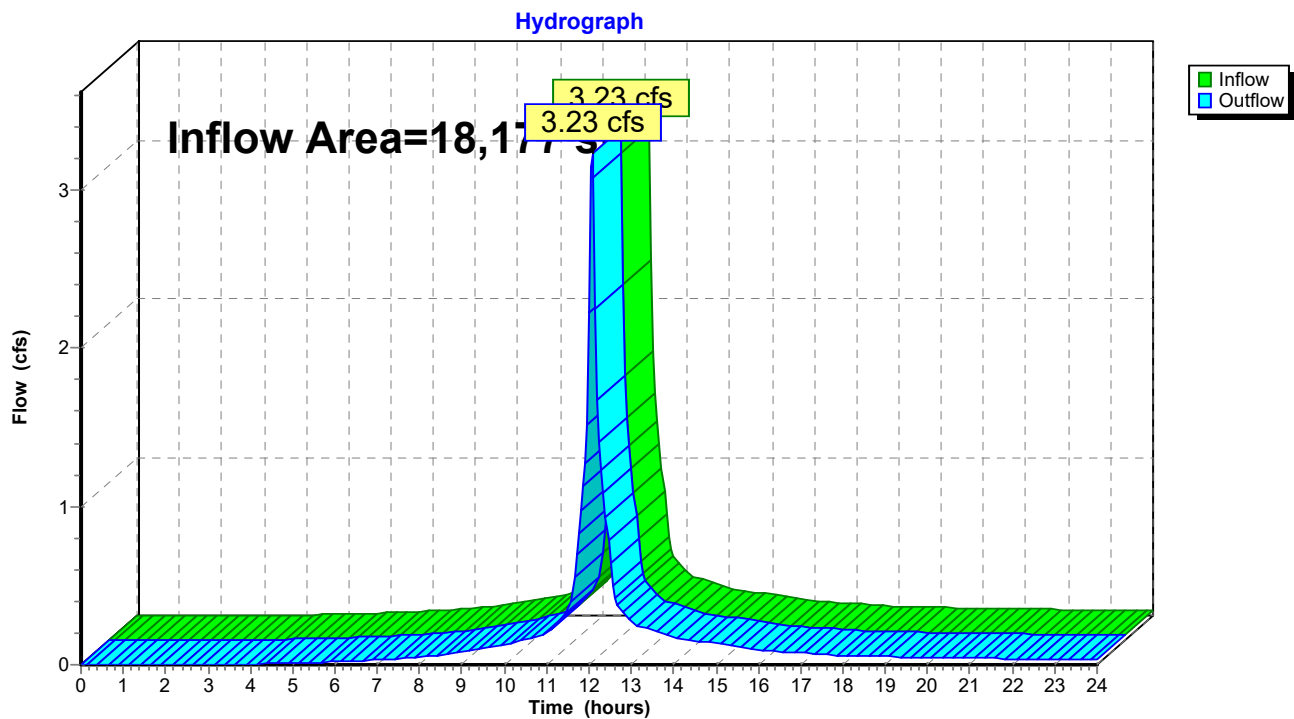
Inflow Area = 18,177 sf, 76.40% Impervious, Inflow Depth > 6.90" for 100-year event

Inflow = 3.23 cfs @ 12.07 hrs, Volume= 10,458 cf

Outflow = 3.23 cfs @ 12.07 hrs, Volume= 10,458 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

### Reach DP-1: Hillside Ave



## Existing Conditions

Prepared by Engineering Alliance Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 100-year Rainfall=8.47"

Printed 10/27/2021

Page 23

### Summary for Reach DP-2: Allenclair Drive

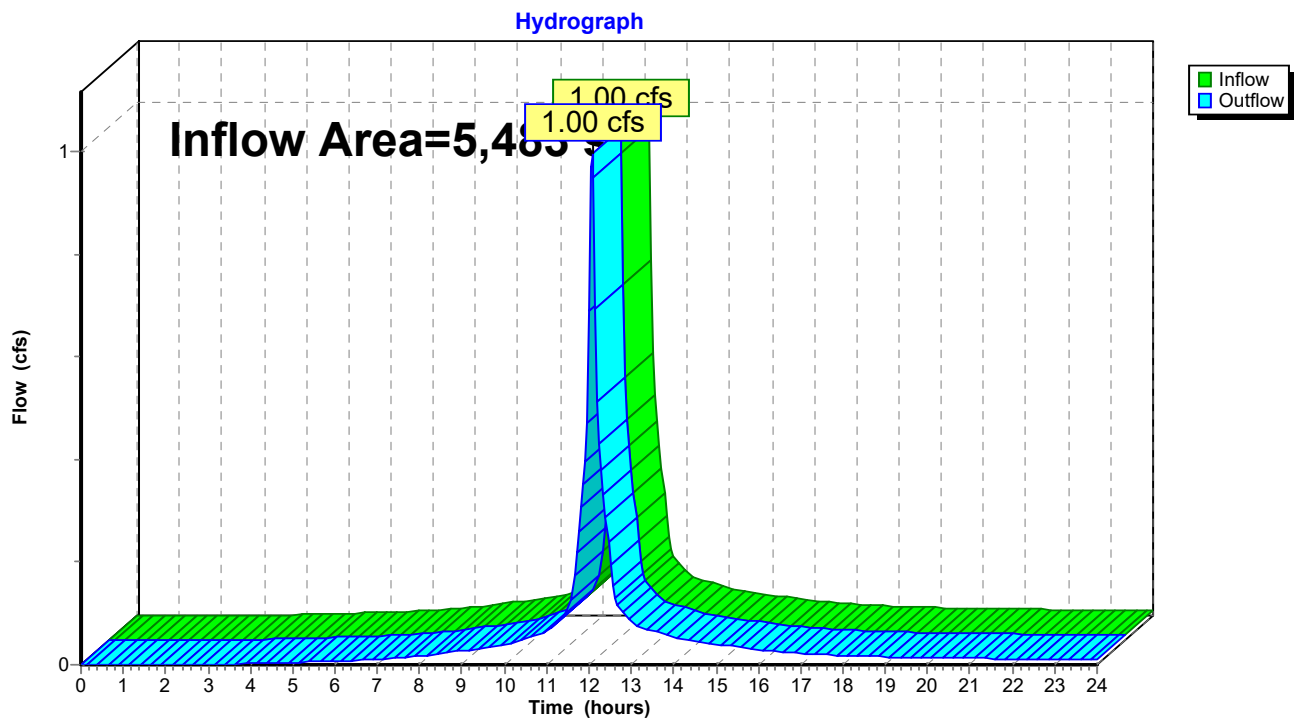
Inflow Area = 5,483 sf, 81.42% Impervious, Inflow Depth > 7.14" for 100-year event

Inflow = 1.00 cfs @ 12.07 hrs, Volume= 3,264 cf

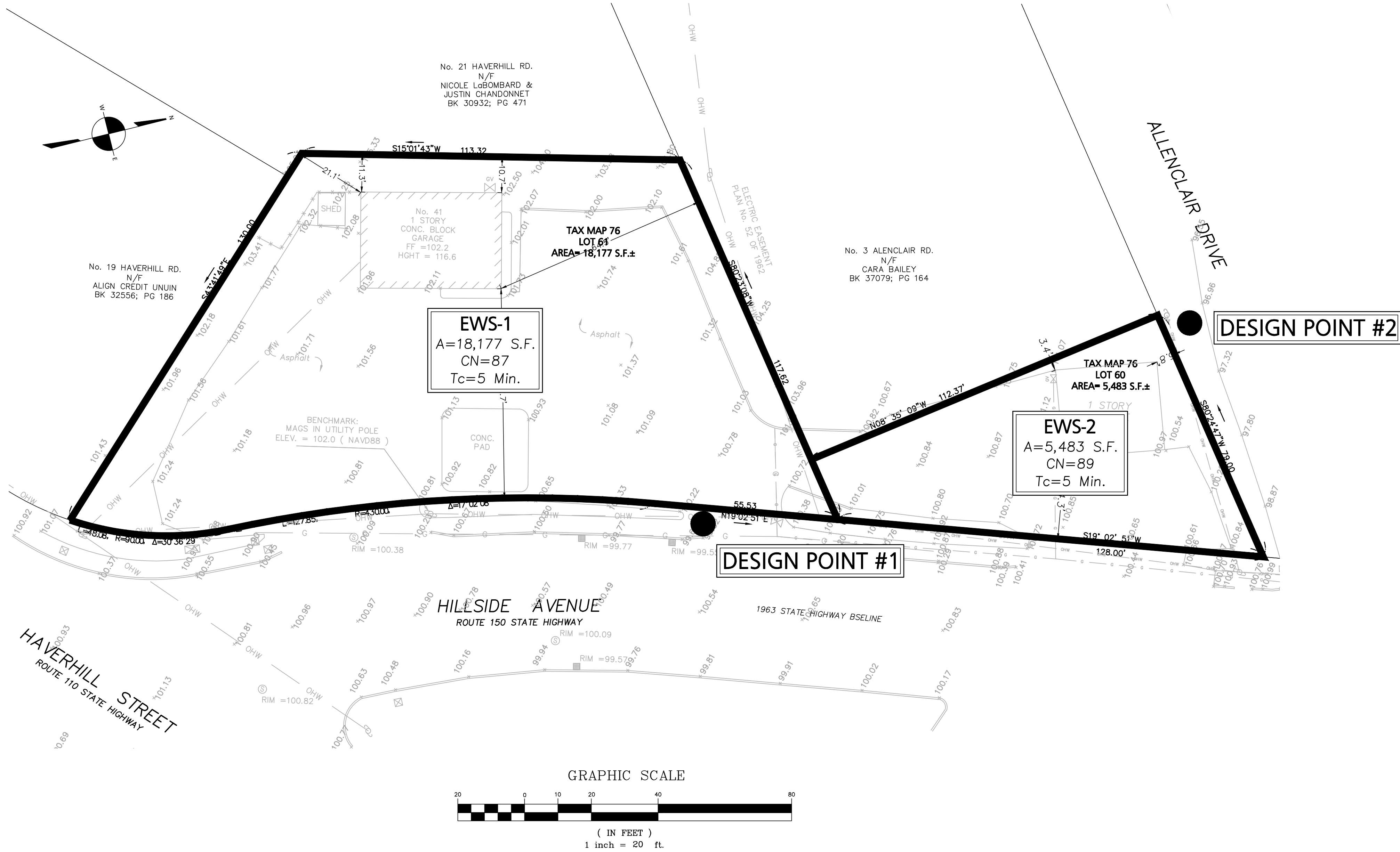
Outflow = 1.00 cfs @ 12.07 hrs, Volume= 3,264 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

### Reach DP-2: Allenclair Drive







LEGEND - EXISTING CONDITIONS PLAN	
PROPERTY LINE	
EXISTING BUILDING	
EXISTING EDGE OF PAVEMENT	
EXISTING CURB	
EXISTING CHAIN LINK FENCE	
EXISTING WATER LINE	
EXISTING WATER VALVE	
EXISTING HYDRANT	
EXISTING SEWER LINE	
EXISTING SEWER MANHOLE	
EXISTING GAS LINE	
EXISTING GAS VALVE	
EXISTING UTILITY POLE	
EXISTING SPOT SHOT	

**GENERAL NOTES**

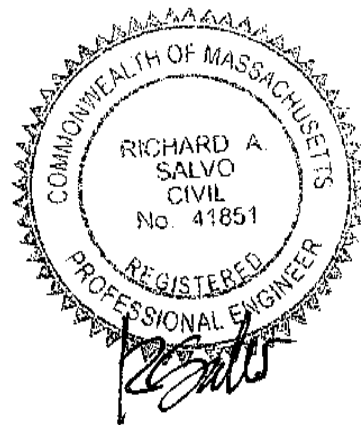
OWNER OF RECORD: ANGIOLILLO MANAGEMENT GROUP INC,  
99 WALNUT STREET  
SAUGUS, MA 01906



DEED REFERENCE: BOOK 37464, PAGE 344  
DEED REFERENCE: BOOK 36487, PAGE 19

TAX MAP REFERENCE: MAP 76 LOTS 60 & 61

EXISTING CONDITIONS INFORMATION OBTAINED FROM AN ACTUAL ON  
THE GROUND SURVEY PREPARED BY BOSTON SURVEY, INC. ON  
MARCH 20, 2021

DATUM REFERENCE: NAVD 88  
ACCORDING TO THE F.E.M.A. MAP FOR ESSEX COUNTY, MAP No.  
25009C0106F, DATED JULY 3, 2012, THE SUBJECT PROPERTY IS LOCATED  
IN A ZONE X.



OWNER :	Angiollilo Management Group Inc. 99 Walnut Street Saugus, MA 01906		DRAWING TITLE: Existing Watershed Plan EWS	
	Site Development Plan 39 & 41 Hillside Avenue Amesbury, Massachusetts			
PROJECT:	PROJECT #:	21-76801	DATE:	October 26, 2021
	SCALE:	AS NOTED	DWG FILE NAME:	21-76801.dwg
PREPARED BY:	DESIGN BY:	Max Friedman	CHECKED BY:	Richard A. Salvo P.E.
	<div><div><div><div>BOSTON</div><div>SURVEY, INC.</div><div>UNIT C-4 SHIPWAYS PLACE CHARLESTOWN, MA 02129 (617) 242-1313 www.bostonsurveyinc.com</div></div></div><div><div><div><div>Engineering Alliance, Inc.</div><div>Civil Engineering &amp; Land Planning Consultants 194 Central Street Saugus, MA 01906 Tel: (781) 231-1349 Fax: (781) 417-0020</div></div></div></div></div>			
PREPARED BY:	DATE			
	DESCRIPTION OF REVISION			

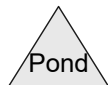
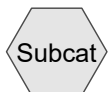
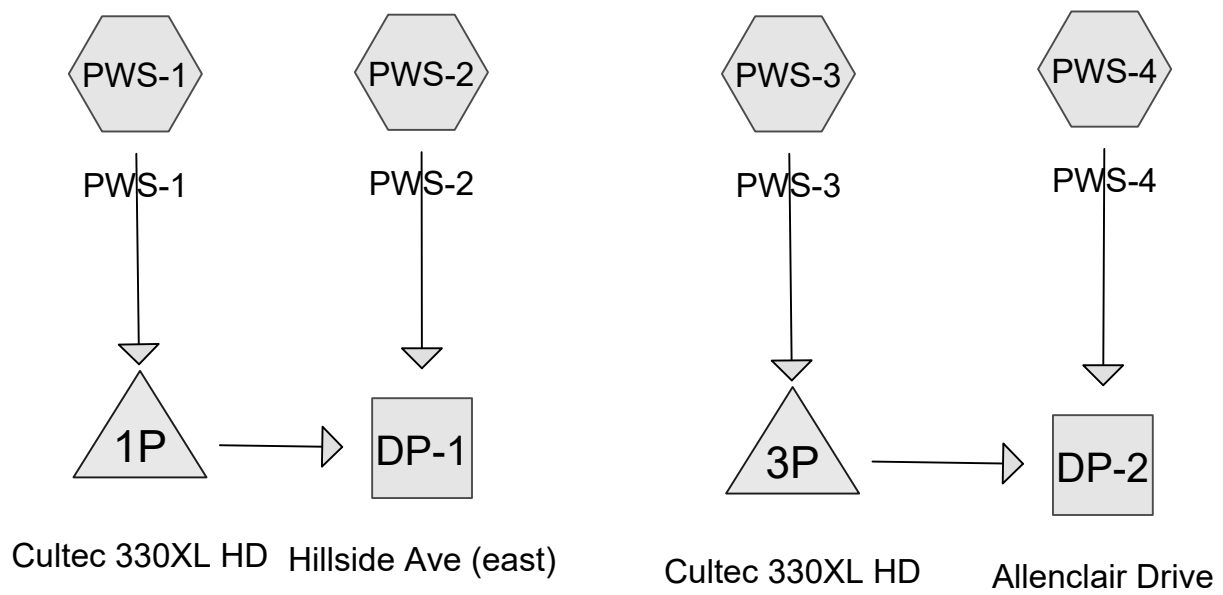


**APPENDIX B**

**Proposed Conditions Drainage Calculations  
Proposed Watershed Plan**







**Proposed Conditions 11-11-21***Type III 24-hr 2-year Rainfall=3.38"*

Prepared by Engineering Alliance, Inc.

Printed 11/18/2021

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Page 2

Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points  
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

**Subcatchment PWS-1: PWS-1** Runoff Area=14,968 sf 88.88% Impervious Runoff Depth>2.43"  
Tc=5.0 min CN=91 Runoff=0.96 cfs 3,026 cf

**Subcatchment PWS-2: PWS-2** Runoff Area=3,444 sf 10.80% Impervious Runoff Depth>0.07"  
Tc=5.0 min CN=45 Runoff=0.00 cfs 19 cf

**Subcatchment PWS-3: PWS-3** Runoff Area=3,881 sf 85.24% Impervious Runoff Depth>2.25"  
Tc=5.0 min CN=89 Runoff=0.23 cfs 726 cf

**Subcatchment PWS-4: PWS-4** Runoff Area=1,367 sf 0.00% Impervious Runoff Depth>0.00"  
Tc=5.0 min CN=39 Runoff=0.00 cfs 0 cf

**Reach DP-1: Hillside Ave (east)** Inflow=0.00 cfs 19 cf  
Outflow=0.00 cfs 19 cf

**Reach DP-2: Allenclair Drive** Inflow=0.00 cfs 0 cf  
Outflow=0.00 cfs 0 cf

**Pond 1P: Cultec 330XL HD** Peak Elev=95.70' Storage=1,003 cf Inflow=0.96 cfs 3,026 cf  
Discarded=0.13 cfs 3,022 cf Primary=0.00 cfs 0 cf Outflow=0.13 cfs 3,022 cf

**Pond 3P: Cultec 330XL HD** Peak Elev=95.88' Storage=222 cf Inflow=0.23 cfs 726 cf  
Discarded=0.04 cfs 725 cf Primary=0.00 cfs 0 cf Outflow=0.04 cfs 725 cf

**Total Runoff Area = 23,660 sf Runoff Volume = 3,772 cf Average Runoff Depth = 1.91"**  
**28.22% Pervious = 6,676 sf 71.78% Impervious = 16,984 sf**

**Proposed Conditions 11-11-21**

Prepared by Engineering Alliance, Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 2-year Rainfall=3.38"

Printed 11/18/2021

Page 3

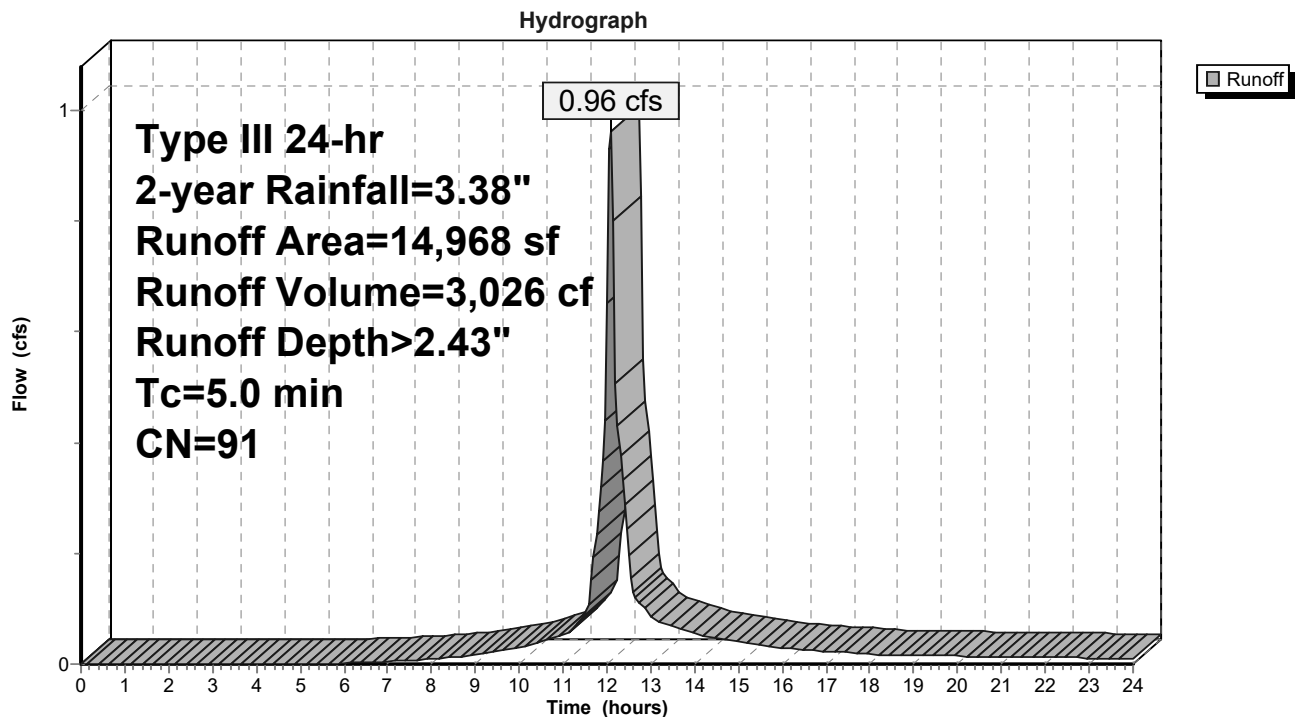
**Summary for Subcatchment PWS-1: PWS-1**

Runoff = 0.96 cfs @ 12.07 hrs, Volume= 3,026 cf, Depth&gt; 2.43"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
Type III 24-hr 2-year Rainfall=3.38"

Area (sf)	CN	Description
8,635	98	Paved parking, HSG A
4,669	98	Roofs, HSG A
1,664	39	>75% Grass cover, Good, HSG A
14,968	91	Weighted Average
1,664		11.12% Pervious Area
13,304		88.88% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

**Subcatchment PWS-1: PWS-1**

**Proposed Conditions 11-11-21**

Prepared by Engineering Alliance, Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 2-year Rainfall=3.38"

Printed 11/18/2021

Page 4

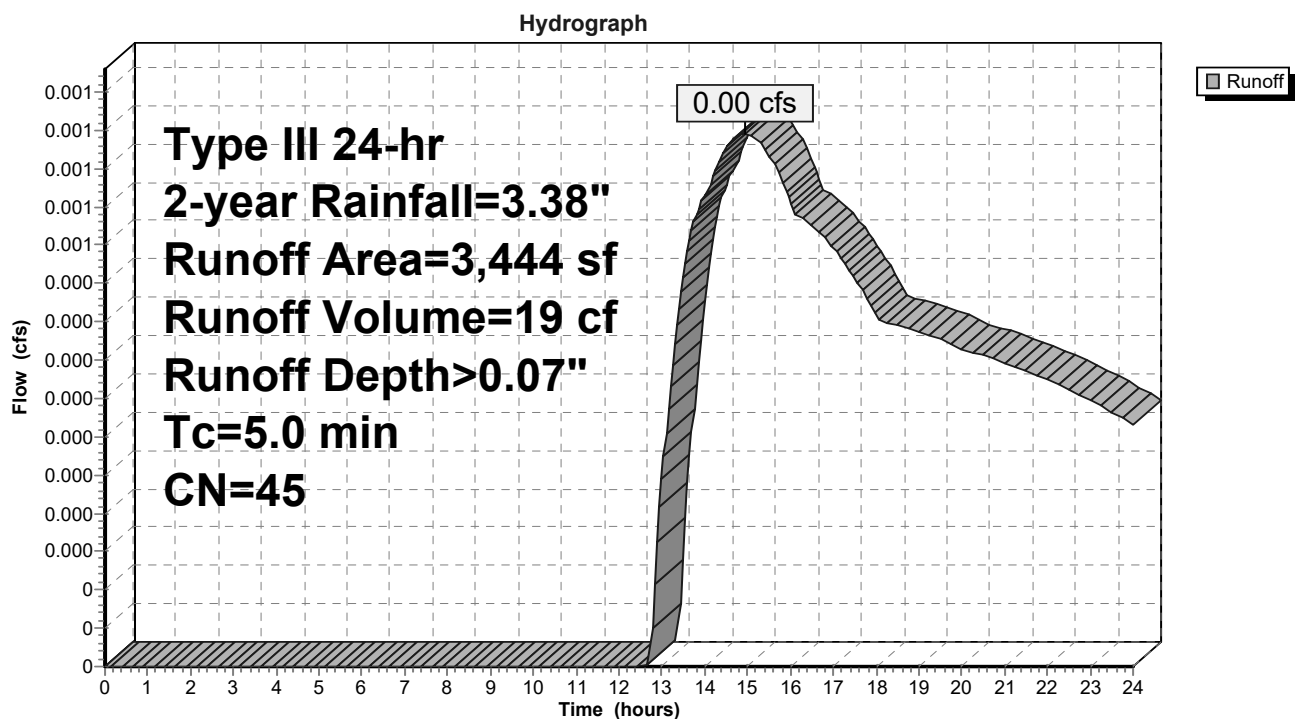
**Summary for Subcatchment PWS-2: PWS-2**

Runoff = 0.00 cfs @ 14.94 hrs, Volume= 19 cf, Depth&gt; 0.07"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
Type III 24-hr 2-year Rainfall=3.38"

Area (sf)	CN	Description
3,072	39	>75% Grass cover, Good, HSG A
372	98	Paved roads w/curbs & sewers, HSG A
3,444	45	Weighted Average
3,072		89.20% Pervious Area
372		10.80% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

**Subcatchment PWS-2: PWS-2**

**Proposed Conditions 11-11-21**

Prepared by Engineering Alliance, Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 2-year Rainfall=3.38"

Printed 11/18/2021

Page 5

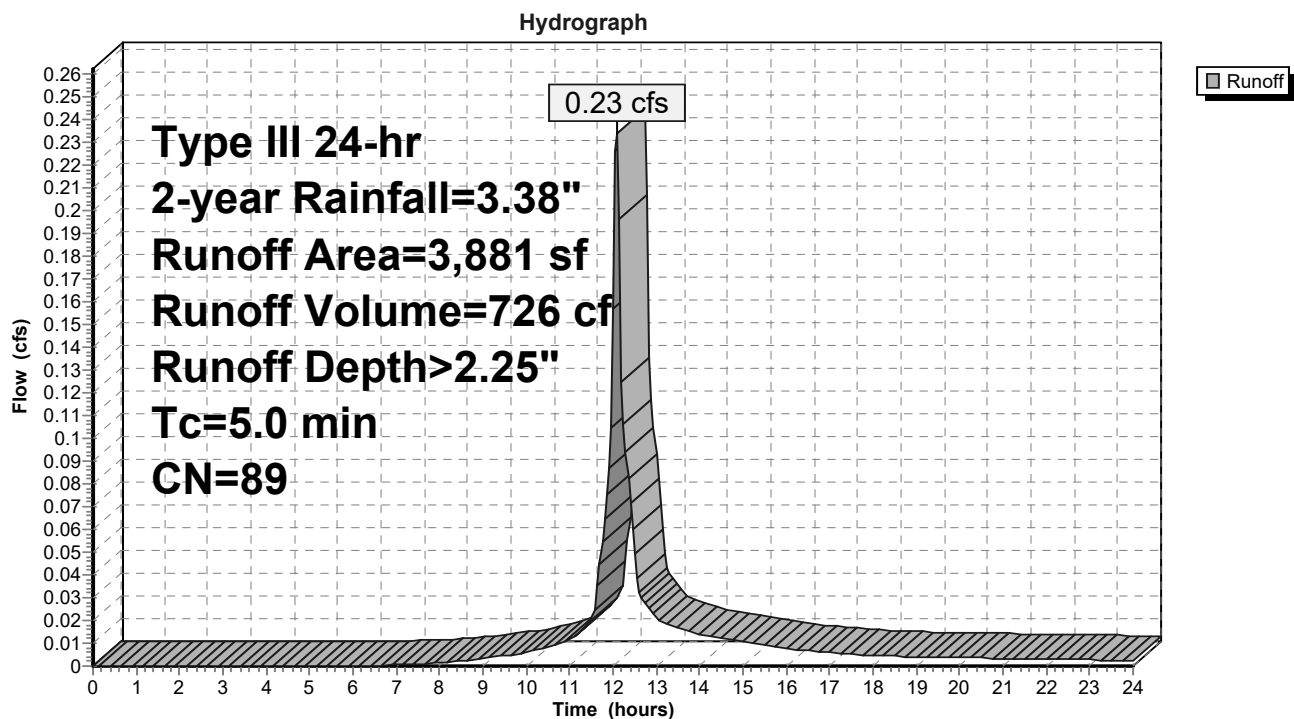
**Summary for Subcatchment PWS-3: PWS-3**

Runoff = 0.23 cfs @ 12.07 hrs, Volume= 726 cf, Depth&gt; 2.25"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
Type III 24-hr 2-year Rainfall=3.38"

Area (sf)	CN	Description
573	39	>75% Grass cover, Good, HSG A
3,290	98	Paved parking, HSG A
* 18	98	Walk, HSG A
3,881	89	Weighted Average
573		14.76% Pervious Area
3,308		85.24% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

**Subcatchment PWS-3: PWS-3**

**Proposed Conditions 11-11-21**

Prepared by Engineering Alliance, Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 2-year Rainfall=3.38"

Printed 11/18/2021

Page 6

**Summary for Subcatchment PWS-4: PWS-4**

Runoff = 0.00 cfs @ 23.64 hrs, Volume= 0 cf, Depth&gt; 0.00"

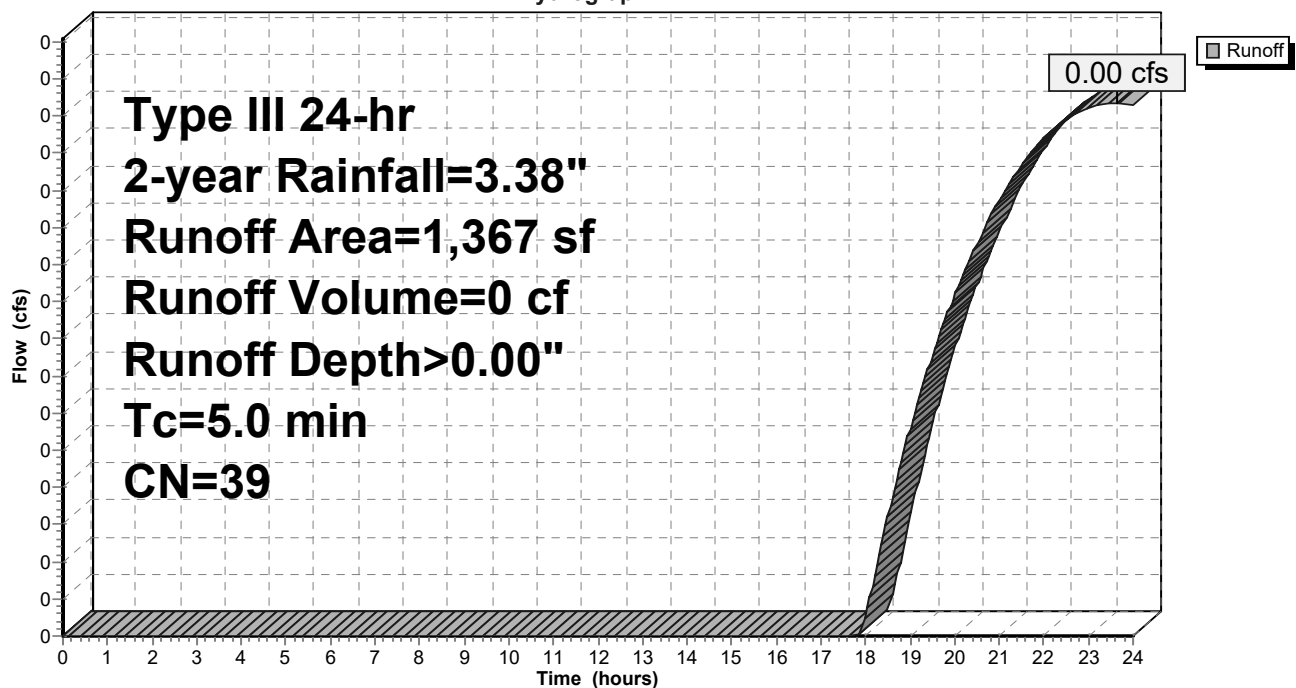
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
Type III 24-hr 2-year Rainfall=3.38"

Area (sf)	CN	Description
1,367	39	>75% Grass cover, Good, HSG A
1,367		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

**Subcatchment PWS-4: PWS-4**

Hydrograph



## Proposed Conditions 11-11-21

Prepared by Engineering Alliance, Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 2-year Rainfall=3.38"

Printed 11/18/2021

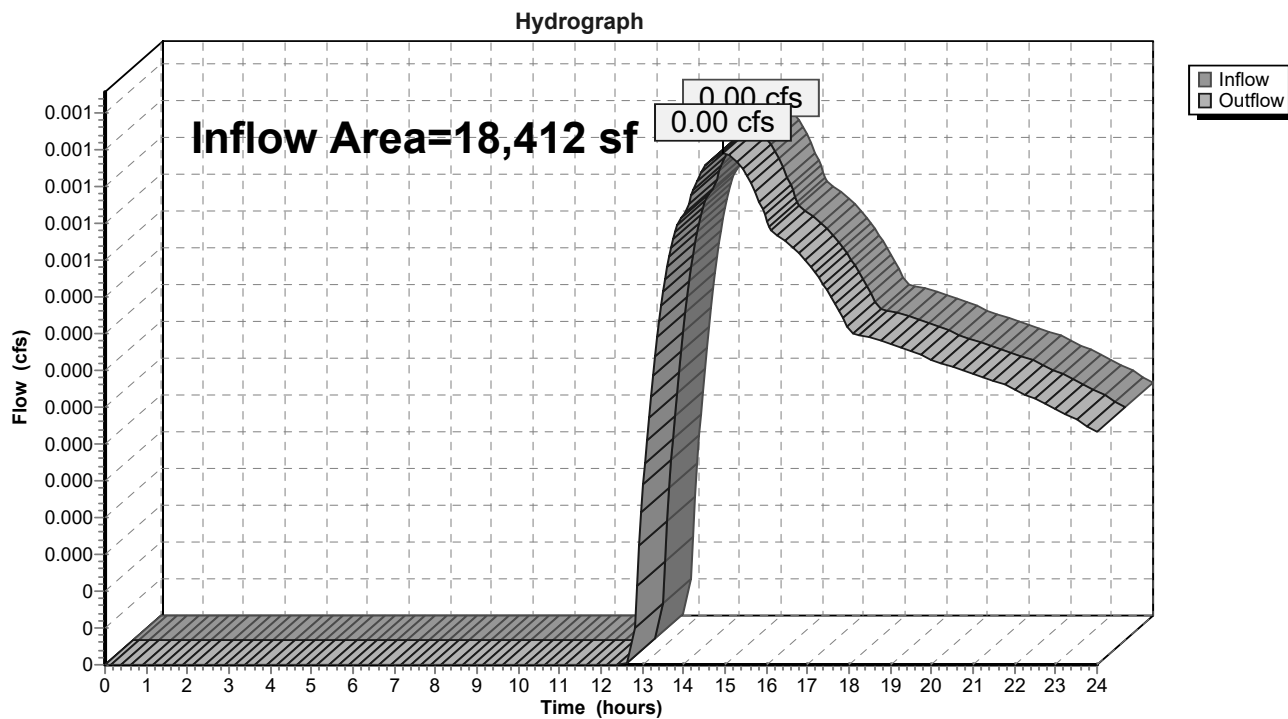
Page 7

### Summary for Reach DP-1: Hillside Ave (east)

Inflow Area = 18,412 sf, 74.28% Impervious, Inflow Depth > 0.01" for 2-year event  
Inflow = 0.00 cfs @ 14.94 hrs, Volume= 19 cf  
Outflow = 0.00 cfs @ 14.94 hrs, Volume= 19 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

### Reach DP-1: Hillside Ave (east)



## Proposed Conditions 11-11-21

Prepared by Engineering Alliance, Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 2-year Rainfall=3.38"

Printed 11/18/2021

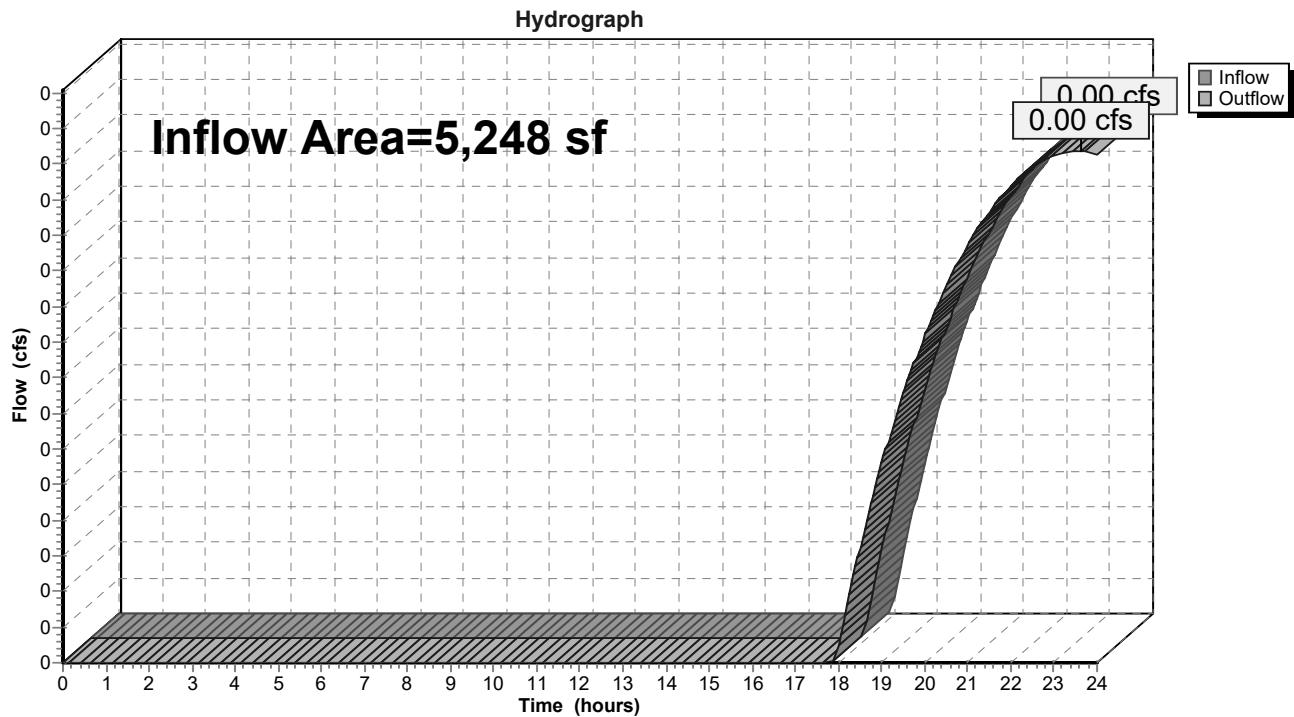
Page 8

### Summary for Reach DP-2: Allenclair Drive

Inflow Area = 5,248 sf, 63.03% Impervious, Inflow Depth > 0.00" for 2-year event  
Inflow = 0.00 cfs @ 23.64 hrs, Volume= 0 cf  
Outflow = 0.00 cfs @ 23.64 hrs, Volume= 0 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

### Reach DP-2: Allenclair Drive





**Proposed Conditions 11-11-21**

Type III 24-hr 2-year Rainfall=3.38"

Prepared by Engineering Alliance, Inc.

Printed 11/18/2021

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Page 9

**Summary for Pond 1P: Cultec 330XL HD**

Inflow Area = 14,968 sf, 88.88% Impervious, Inflow Depth > 2.43" for 2-year event  
 Inflow = 0.96 cfs @ 12.07 hrs, Volume= 3,026 cf  
 Outflow = 0.13 cfs @ 11.70 hrs, Volume= 3,022 cf, Atten= 87%, Lag= 0.0 min  
 Discarded = 0.13 cfs @ 11.70 hrs, Volume= 3,022 cf  
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2  
 Peak Elev= 95.70' @ 12.63 hrs Surf.Area= 2,242 sf Storage= 1,003 cf

Plug-Flow detention time= 57.8 min calculated for 3,022 cf (100% of inflow)  
 Center-of-Mass det. time= 56.9 min ( 856.0 - 799.1 )

Volume	Invert	Avail.Storage	Storage Description
#1A	94.91'	1,897 cf	<b>30.50'W x 73.50'L x 3.54'H Field A</b> 7,940 cf Overall - 3,196 cf Embedded = 4,743 cf x 40.0% Voids
#2A	95.41'	3,196 cf	<b>Cultec R-330XLHD x 60 Inside #1</b> Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 6 rows
#3	98.45'	0 cf	<b>0.50'D x 1.77'H Vertical Cone/Cylinder-Impervious</b>
		5,094 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	94.91'	<b>2.410 in/hr Exfiltration over Surface area</b>
#2	Primary	100.21'	<b>6.0" Vert. Orifice/Grate X 3.00</b> C= 0.600 Limited to weir flow at low heads

**Discarded OutFlow** Max=0.13 cfs @ 11.70 hrs HW=94.97' (Free Discharge)  
 ↑**1=Exfiltration** (Exfiltration Controls 0.13 cfs)

**Primary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=94.91' (Free Discharge)  
 ↑**2=Orifice/Grate** ( Controls 0.00 cfs)

## Proposed Conditions 11-11-21

Prepared by Engineering Alliance, Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 2-year Rainfall=3.38"

Printed 11/18/2021

Page 10

### Pond 1P: Cultec 330XL HD - Chamber Wizard Field A

#### Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf

Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap

Row Length Adjustment= +1.50' x 7.45 sf x 6 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

10 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 71.50' Row Length +12.0" End Stone x 2 = 73.50' Base Length

6 Rows x 52.0" Wide + 6.0" Spacing x 5 + 12.0" Side Stone x 2 = 30.50' Base Width

6.0" Stone Base + 30.5" Chamber Height + 6.0" Stone Cover = 3.54' Field Height

60 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 6 Rows = 3,196.5 cf Chamber Storage

7,939.5 cf Field - 3,196.5 cf Chambers = 4,743.1 cf Stone x 40.0% Voids = 1,897.2 cf Stone Storage

Chamber Storage + Stone Storage = 5,093.7 cf = 0.117 af

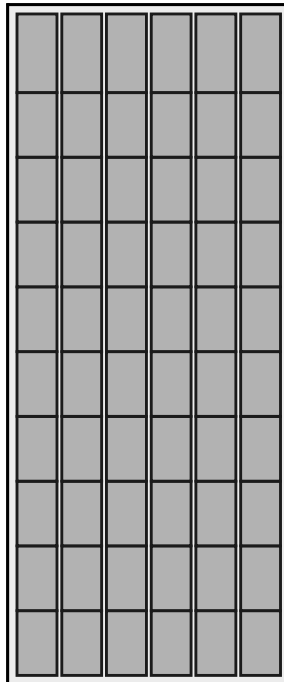
Overall Storage Efficiency = 64.2%

Overall System Size = 73.50' x 30.50' x 3.54'

60 Chambers

294.1 cy Field

175.7 cy Stone



# Proposed Conditions 11-11-21

Prepared by Engineering Alliance, Inc.

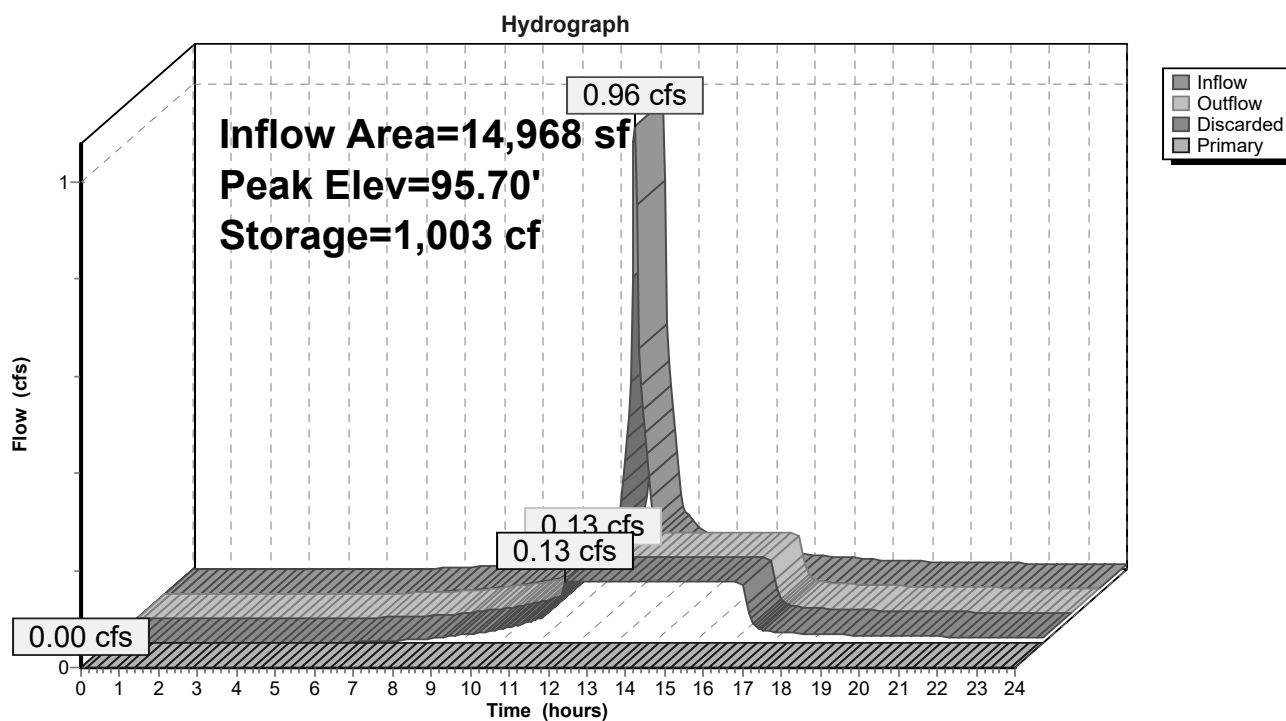
HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 2-year Rainfall=3.38"

Printed 11/18/2021

Page 11

## Pond 1P: Cultec 330XL HD



**Proposed Conditions 11-11-21**

Type III 24-hr 2-year Rainfall=3.38"

Prepared by Engineering Alliance, Inc.

Printed 11/18/2021

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Page 12

**Summary for Pond 3P: Cultec 330XL HD**

Inflow Area = 3,881 sf, 85.24% Impervious, Inflow Depth > 2.25" for 2-year event  
 Inflow = 0.23 cfs @ 12.07 hrs, Volume= 726 cf  
 Outflow = 0.04 cfs @ 11.75 hrs, Volume= 725 cf, Atten= 84%, Lag= 0.0 min  
 Discarded = 0.04 cfs @ 11.75 hrs, Volume= 725 cf  
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2

Peak Elev= 95.88' @ 12.57 hrs Surf.Area= 656 sf Storage= 222 cf

Plug-Flow detention time= 41.4 min calculated for 724 cf (100% of inflow)

Center-of-Mass det. time= 40.7 min ( 848.3 - 807.6 )

Volume	Invert	Avail.Storage	Storage Description
#1A	95.21'	578 cf	<b>20.83'W x 31.50'L x 3.54'H Field A</b> 2,324 cf Overall - 879 cf Embedded = 1,445 cf x 40.0% Voids
#2A	95.71'	879 cf	<b>Cultec R-330XLHD x 16 Inside #1</b> Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 4 rows
#3	98.80'	0 cf	<b>0.50'D x 1.08'H Vertical Cone/Cylinder-Impervious</b>
		1,457 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	95.21'	<b>2.410 in/hr Exfiltration over Surface area</b>
#2	Primary	99.83'	<b>6.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Discarded OutFlow** Max=0.04 cfs @ 11.75 hrs HW=95.26' (Free Discharge)↑**1=Exfiltration** (Exfiltration Controls 0.04 cfs)**Primary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=95.21' (Free Discharge)↑**2=Orifice/Grate** ( Controls 0.00 cfs)

## Proposed Conditions 11-11-21

Prepared by Engineering Alliance, Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 2-year Rainfall=3.38"

Printed 11/18/2021

Page 13

### Pond 3P: Cultec 330XL HD - Chamber Wizard Field A

#### Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf

Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap

Row Length Adjustment= +1.50' x 7.45 sf x 4 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

4 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 29.50' Row Length +12.0" End Stone x 2 = 31.50' Base Length

4 Rows x 52.0" Wide + 6.0" Spacing x 3 + 12.0" Side Stone x 2 = 20.83' Base Width

6.0" Stone Base + 30.5" Chamber Height + 6.0" Stone Cover = 3.54' Field Height

16 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 4 Rows = 879.2 cf Chamber Storage

2,324.2 cf Field - 879.2 cf Chambers = 1,445.0 cf Stone x 40.0% Voids = 578.0 cf Stone Storage

Chamber Storage + Stone Storage = 1,457.2 cf = 0.033 af

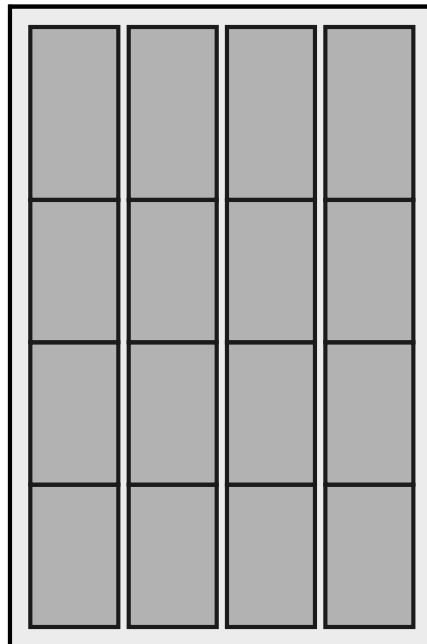
Overall Storage Efficiency = 62.7%

Overall System Size = 31.50' x 20.83' x 3.54'

16 Chambers

86.1 cy Field

53.5 cy Stone



# Proposed Conditions 11-11-21

Prepared by Engineering Alliance, Inc.

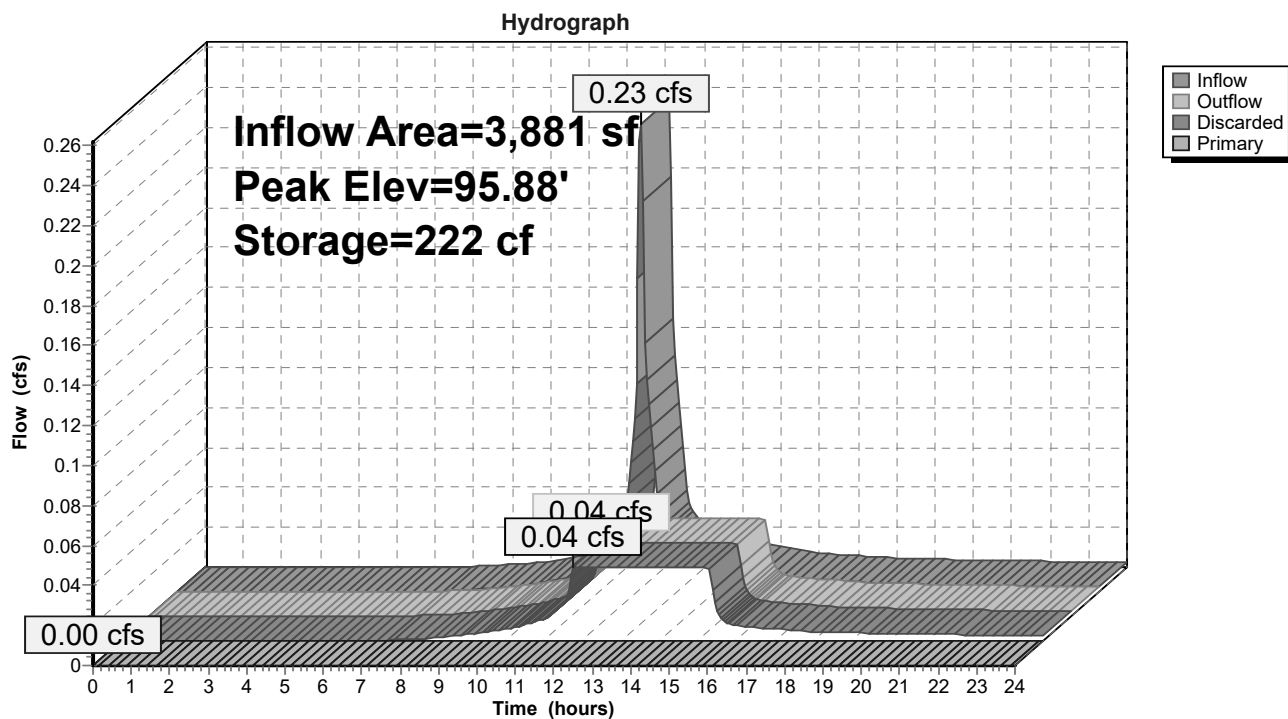
HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 2-year Rainfall=3.38"

Printed 11/18/2021

Page 14

## Pond 3P: Cultec 330XL HD



**Proposed Conditions 11-11-21***Type III 24-hr 10-year Rainfall=5.35"*

Prepared by Engineering Alliance, Inc.

Printed 11/18/2021

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Page 15

Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points  
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

<b>Subcatchment PWS-1: PWS-1</b>	Runoff Area=14,968 sf 88.88% Impervious Runoff Depth>4.32" Tc=5.0 min CN=91 Runoff=1.67 cfs 5,389 cf
<b>Subcatchment PWS-2: PWS-2</b>	Runoff Area=3,444 sf 10.80% Impervious Runoff Depth>0.56" Tc=5.0 min CN=45 Runoff=0.02 cfs 160 cf
<b>Subcatchment PWS-3: PWS-3</b>	Runoff Area=3,881 sf 85.24% Impervious Runoff Depth>4.11" Tc=5.0 min CN=89 Runoff=0.42 cfs 1,328 cf
<b>Subcatchment PWS-4: PWS-4</b>	Runoff Area=1,367 sf 0.00% Impervious Runoff Depth>0.28" Tc=5.0 min CN=39 Runoff=0.00 cfs 31 cf
<b>Reach DP-1: Hillside Ave (east)</b>	Inflow=0.02 cfs 160 cf Outflow=0.02 cfs 160 cf
<b>Reach DP-2: Allenclair Drive</b>	Inflow=0.00 cfs 31 cf Outflow=0.00 cfs 31 cf
<b>Pond 1P: Cultec 330XL HD</b>	Peak Elev=96.33' Storage=2,171 cf Inflow=1.67 cfs 5,389 cf Discarded=0.13 cfs 5,382 cf Primary=0.00 cfs 0 cf Outflow=0.13 cfs 5,382 cf
<b>Pond 3P: Cultec 330XL HD</b>	Peak Elev=96.41' Storage=508 cf Inflow=0.42 cfs 1,328 cf Discarded=0.04 cfs 1,326 cf Primary=0.00 cfs 0 cf Outflow=0.04 cfs 1,326 cf
<b>Total Runoff Area = 23,660 sf Runoff Volume = 6,908 cf Average Runoff Depth = 3.50"</b> <b>28.22% Pervious = 6,676 sf 71.78% Impervious = 16,984 sf</b>	

**Proposed Conditions 11-11-21**

Prepared by Engineering Alliance, Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 10-year Rainfall=5.35"

Printed 11/18/2021

Page 16

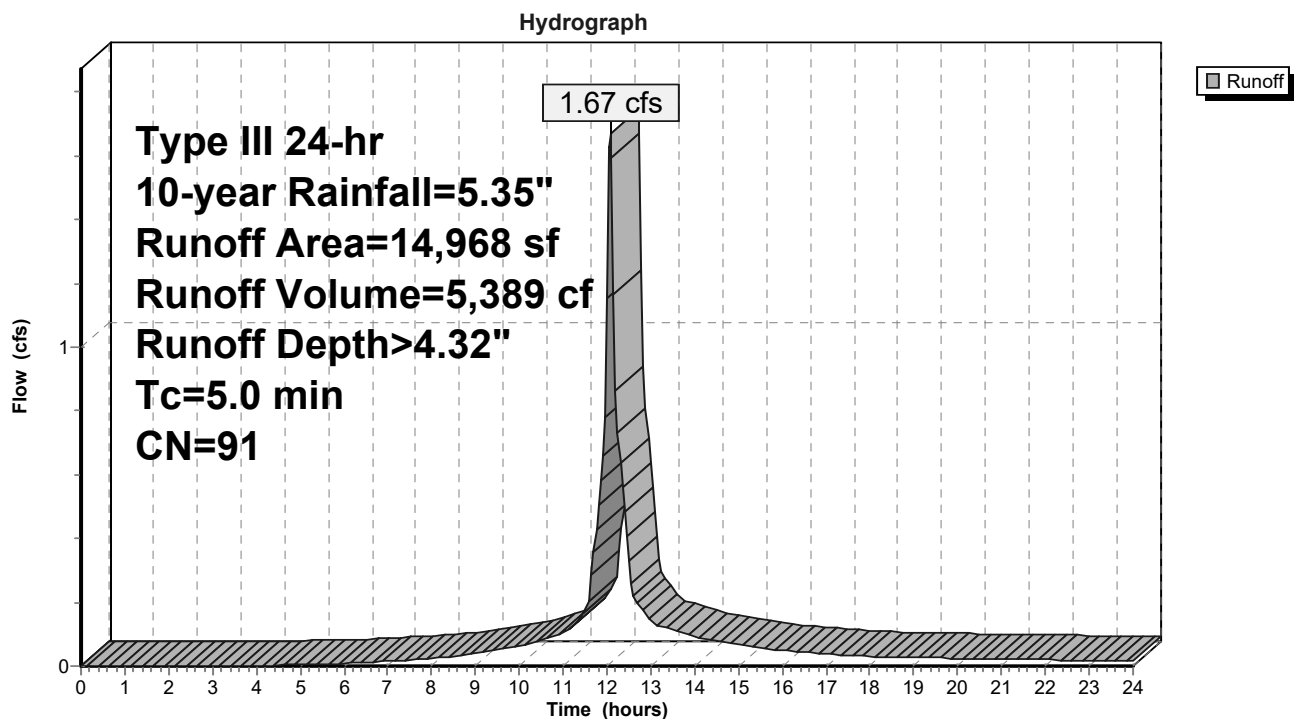
**Summary for Subcatchment PWS-1: PWS-1**

Runoff = 1.67 cfs @ 12.07 hrs, Volume= 5,389 cf, Depth&gt; 4.32"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
Type III 24-hr 10-year Rainfall=5.35"

Area (sf)	CN	Description
8,635	98	Paved parking, HSG A
4,669	98	Roofs, HSG A
1,664	39	>75% Grass cover, Good, HSG A
14,968	91	Weighted Average
1,664		11.12% Pervious Area
13,304		88.88% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

**Subcatchment PWS-1: PWS-1**



**Proposed Conditions 11-11-21**

Prepared by Engineering Alliance, Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 10-year Rainfall=5.35"

Printed 11/18/2021

Page 17

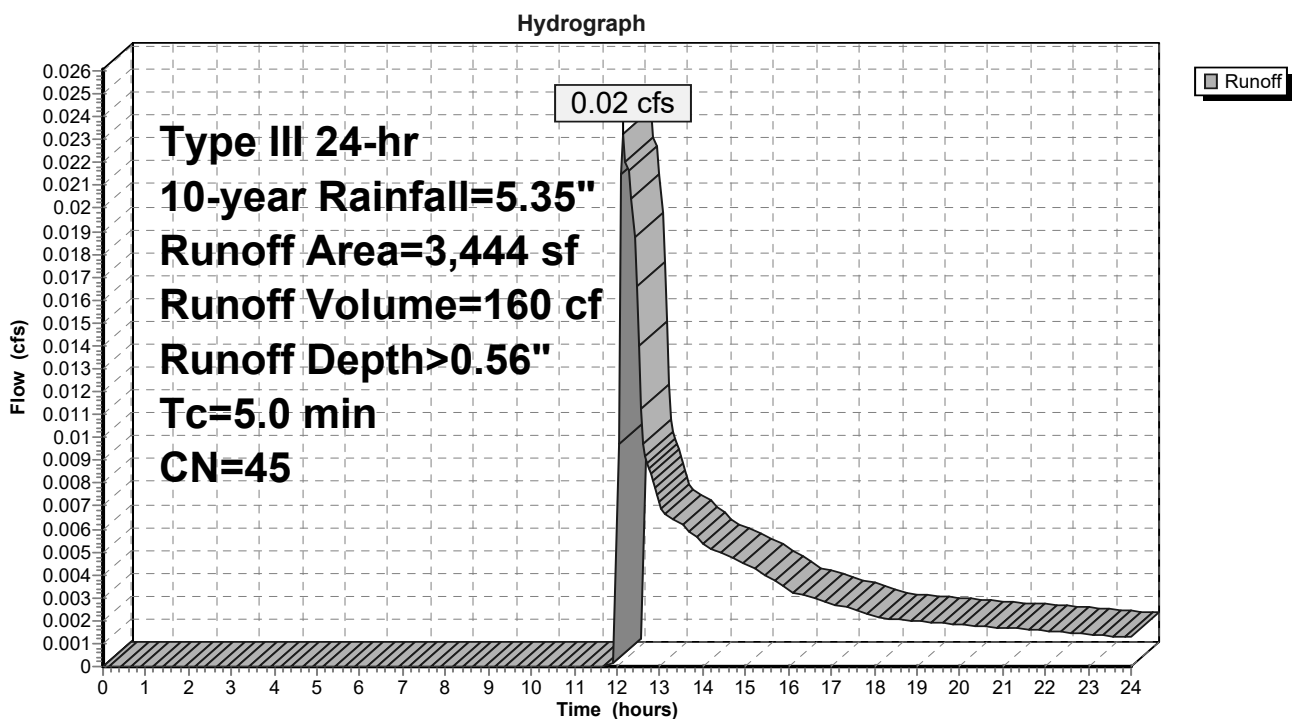
**Summary for Subcatchment PWS-2: PWS-2**

Runoff = 0.02 cfs @ 12.15 hrs, Volume= 160 cf, Depth&gt; 0.56"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
Type III 24-hr 10-year Rainfall=5.35"

Area (sf)	CN	Description
3,072	39	>75% Grass cover, Good, HSG A
372	98	Paved roads w/curbs & sewers, HSG A
3,444	45	Weighted Average
3,072		89.20% Pervious Area
372		10.80% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

**Subcatchment PWS-2: PWS-2**

**Proposed Conditions 11-11-21**

Prepared by Engineering Alliance, Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 10-year Rainfall=5.35"

Printed 11/18/2021

Page 18

**Summary for Subcatchment PWS-3: PWS-3**

Runoff = 0.42 cfs @ 12.07 hrs, Volume= 1,328 cf, Depth&gt; 4.11"

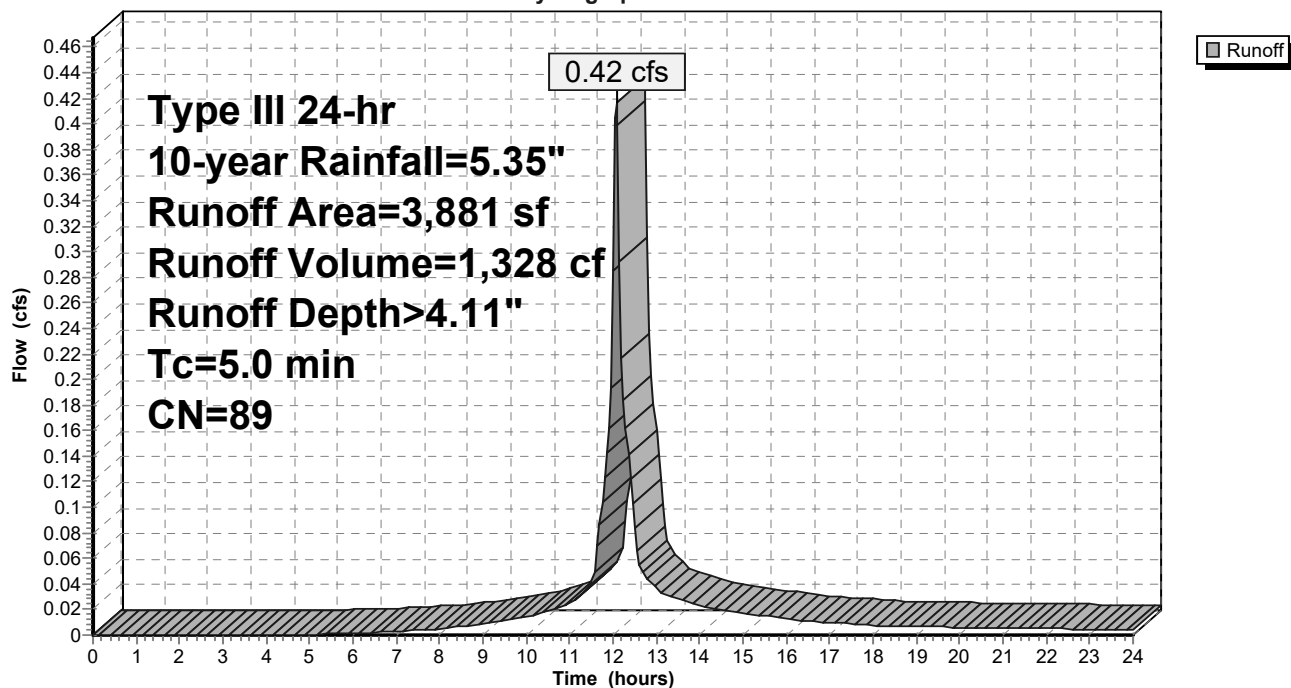
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
Type III 24-hr 10-year Rainfall=5.35"

Area (sf)	CN	Description
573	39	>75% Grass cover, Good, HSG A
3,290	98	Paved parking, HSG A
* 18	98	Walk, HSG A
3,881	89	Weighted Average
573		14.76% Pervious Area
3,308		85.24% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

**Subcatchment PWS-3: PWS-3**

Hydrograph



**Proposed Conditions 11-11-21**

Prepared by Engineering Alliance, Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 10-year Rainfall=5.35"

Printed 11/18/2021

Page 19

**Summary for Subcatchment PWS-4: PWS-4**

Runoff = 0.00 cfs @ 12.40 hrs, Volume= 31 cf, Depth&gt; 0.28"

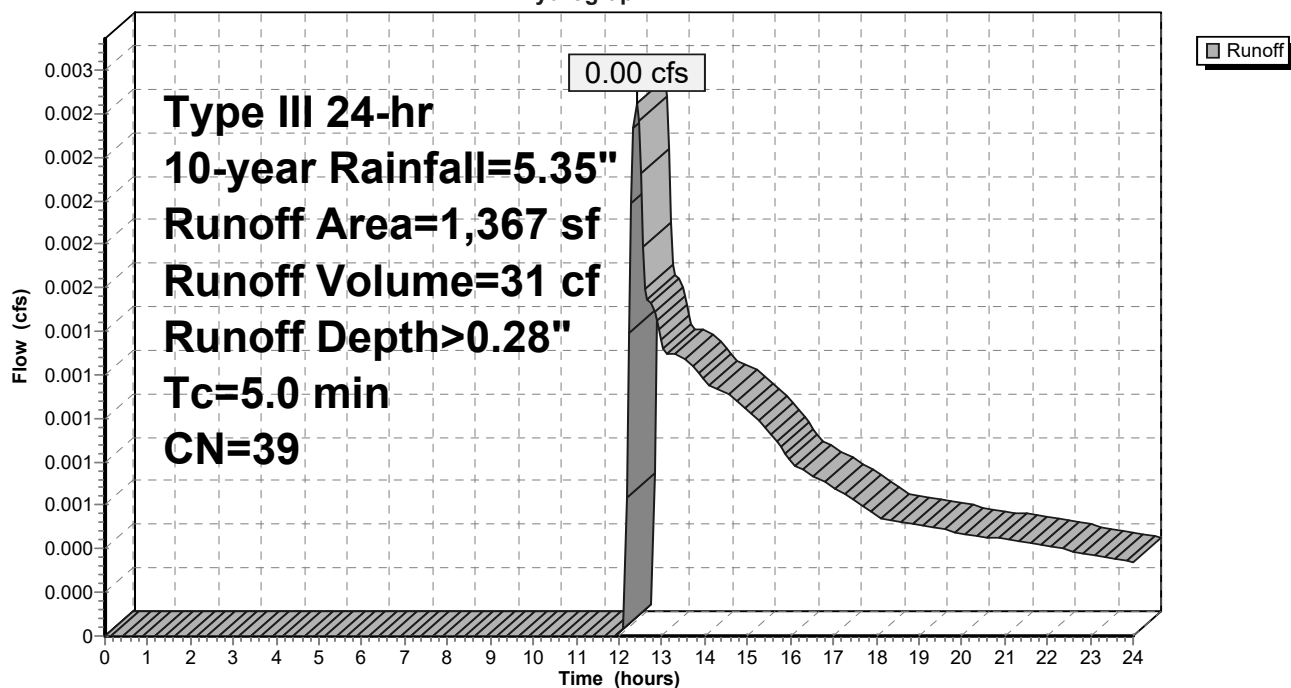
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
Type III 24-hr 10-year Rainfall=5.35"

Area (sf)	CN	Description
1,367	39	>75% Grass cover, Good, HSG A
1,367		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

**Subcatchment PWS-4: PWS-4**

Hydrograph



## Proposed Conditions 11-11-21

Prepared by Engineering Alliance, Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 10-year Rainfall=5.35"

Printed 11/18/2021

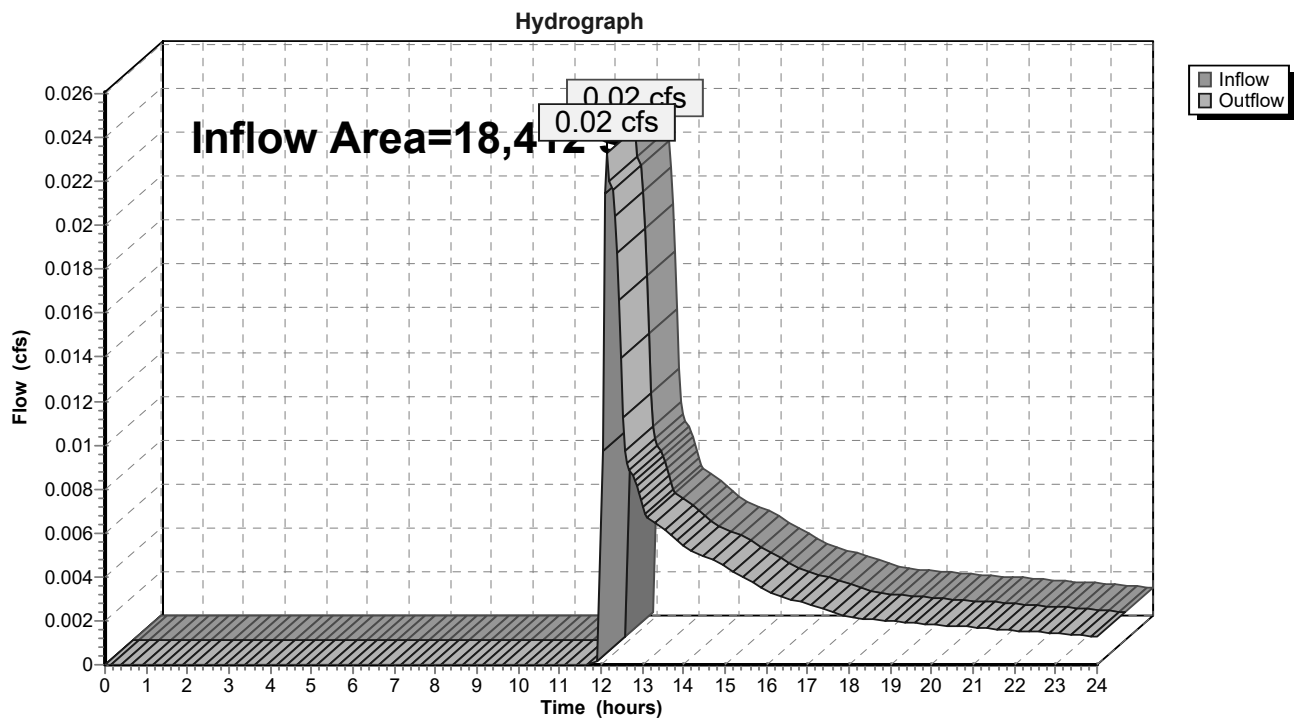
Page 20

### Summary for Reach DP-1: Hillside Ave (east)

Inflow Area = 18,412 sf, 74.28% Impervious, Inflow Depth > 0.10" for 10-year event  
Inflow = 0.02 cfs @ 12.15 hrs, Volume= 160 cf  
Outflow = 0.02 cfs @ 12.15 hrs, Volume= 160 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

### Reach DP-1: Hillside Ave (east)



## Proposed Conditions 11-11-21

Prepared by Engineering Alliance, Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 10-year Rainfall=5.35"

Printed 11/18/2021

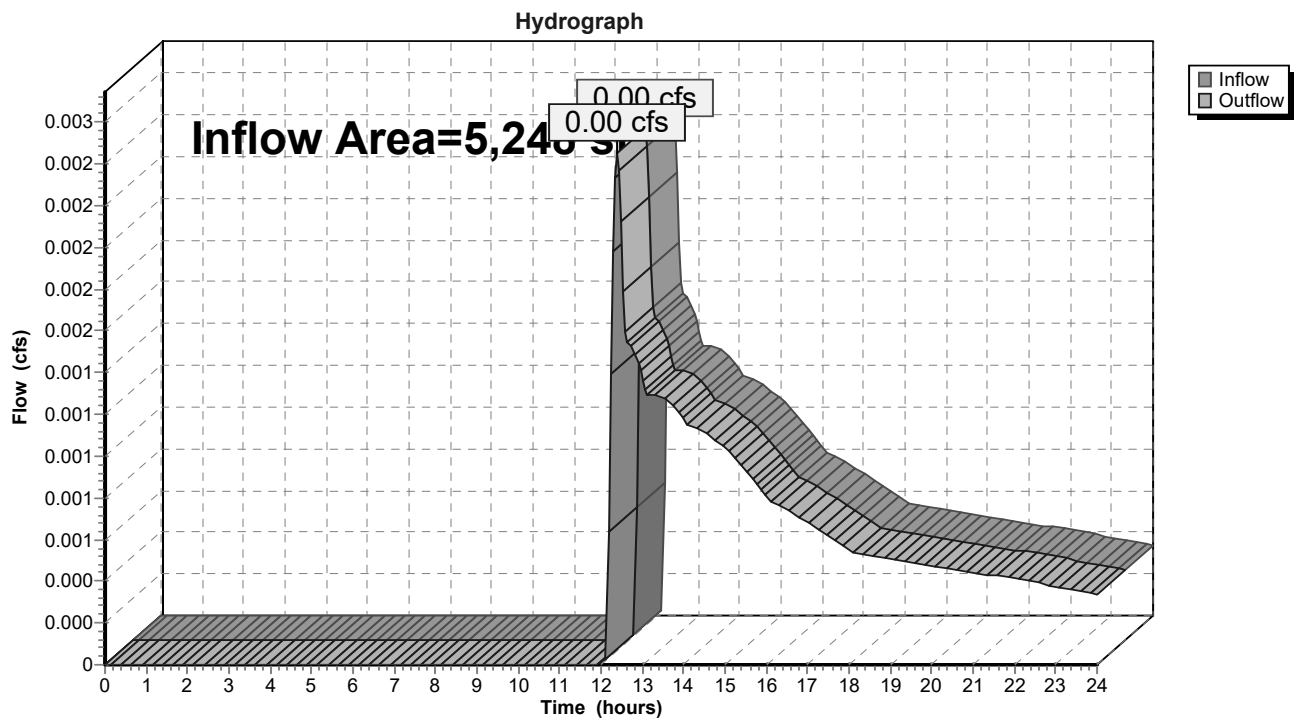
Page 21

### Summary for Reach DP-2: Allenclair Drive

Inflow Area = 5,248 sf, 63.03% Impervious, Inflow Depth > 0.07" for 10-year event  
Inflow = 0.00 cfs @ 12.40 hrs, Volume= 31 cf  
Outflow = 0.00 cfs @ 12.40 hrs, Volume= 31 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

### Reach DP-2: Allenclair Drive



**Proposed Conditions 11-11-21**

Type III 24-hr 10-year Rainfall=5.35"

Prepared by Engineering Alliance, Inc.

Printed 11/18/2021

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Page 22

**Summary for Pond 1P: Cultec 330XL HD**

Inflow Area = 14,968 sf, 88.88% Impervious, Inflow Depth > 4.32" for 10-year event  
 Inflow = 1.67 cfs @ 12.07 hrs, Volume= 5,389 cf  
 Outflow = 0.13 cfs @ 11.35 hrs, Volume= 5,382 cf, Atten= 93%, Lag= 0.0 min  
 Discarded = 0.13 cfs @ 11.35 hrs, Volume= 5,382 cf  
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2  
 Peak Elev= 96.33' @ 13.19 hrs Surf.Area= 2,242 sf Storage= 2,171 cf

Plug-Flow detention time= 141.1 min calculated for 5,382 cf (100% of inflow)  
 Center-of-Mass det. time= 140.3 min ( 923.7 - 783.4 )

Volume	Invert	Avail.Storage	Storage Description
#1A	94.91'	1,897 cf	<b>30.50'W x 73.50'L x 3.54'H Field A</b> 7,940 cf Overall - 3,196 cf Embedded = 4,743 cf x 40.0% Voids
#2A	95.41'	3,196 cf	<b>Cultec R-330XLHD x 60 Inside #1</b> Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 6 rows
#3	98.45'	0 cf	<b>0.50'D x 1.77'H Vertical Cone/Cylinder-Impervious</b>
		5,094 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	94.91'	<b>2.410 in/hr Exfiltration over Surface area</b>
#2	Primary	100.21'	<b>6.0" Vert. Orifice/Grate X 3.00</b> C= 0.600 Limited to weir flow at low heads

**Discarded OutFlow** Max=0.13 cfs @ 11.35 hrs HW=94.97' (Free Discharge)  
 ↑**1=Exfiltration** (Exfiltration Controls 0.13 cfs)

**Primary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=94.91' (Free Discharge)  
 ↑**2=Orifice/Grate** ( Controls 0.00 cfs)

## Proposed Conditions 11-11-21

Prepared by Engineering Alliance, Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 10-year Rainfall=5.35"

Printed 11/18/2021

Page 23

### Pond 1P: Cultec 330XL HD - Chamber Wizard Field A

#### Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf

Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap

Row Length Adjustment= +1.50' x 7.45 sf x 6 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

10 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 71.50' Row Length +12.0" End Stone x 2 = 73.50' Base Length

6 Rows x 52.0" Wide + 6.0" Spacing x 5 + 12.0" Side Stone x 2 = 30.50' Base Width

6.0" Stone Base + 30.5" Chamber Height + 6.0" Stone Cover = 3.54' Field Height

60 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 6 Rows = 3,196.5 cf Chamber Storage

7,939.5 cf Field - 3,196.5 cf Chambers = 4,743.1 cf Stone x 40.0% Voids = 1,897.2 cf Stone Storage

Chamber Storage + Stone Storage = 5,093.7 cf = 0.117 af

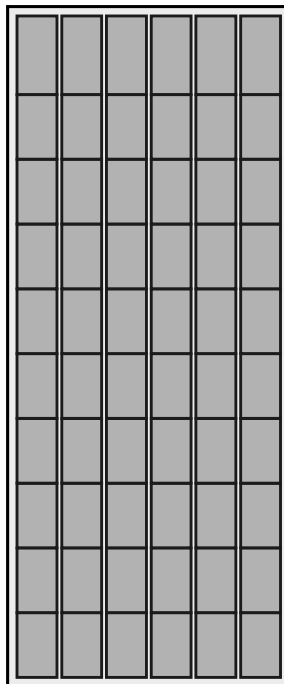
Overall Storage Efficiency = 64.2%

Overall System Size = 73.50' x 30.50' x 3.54'

60 Chambers

294.1 cy Field

175.7 cy Stone



# Proposed Conditions 11-11-21

Prepared by Engineering Alliance, Inc.

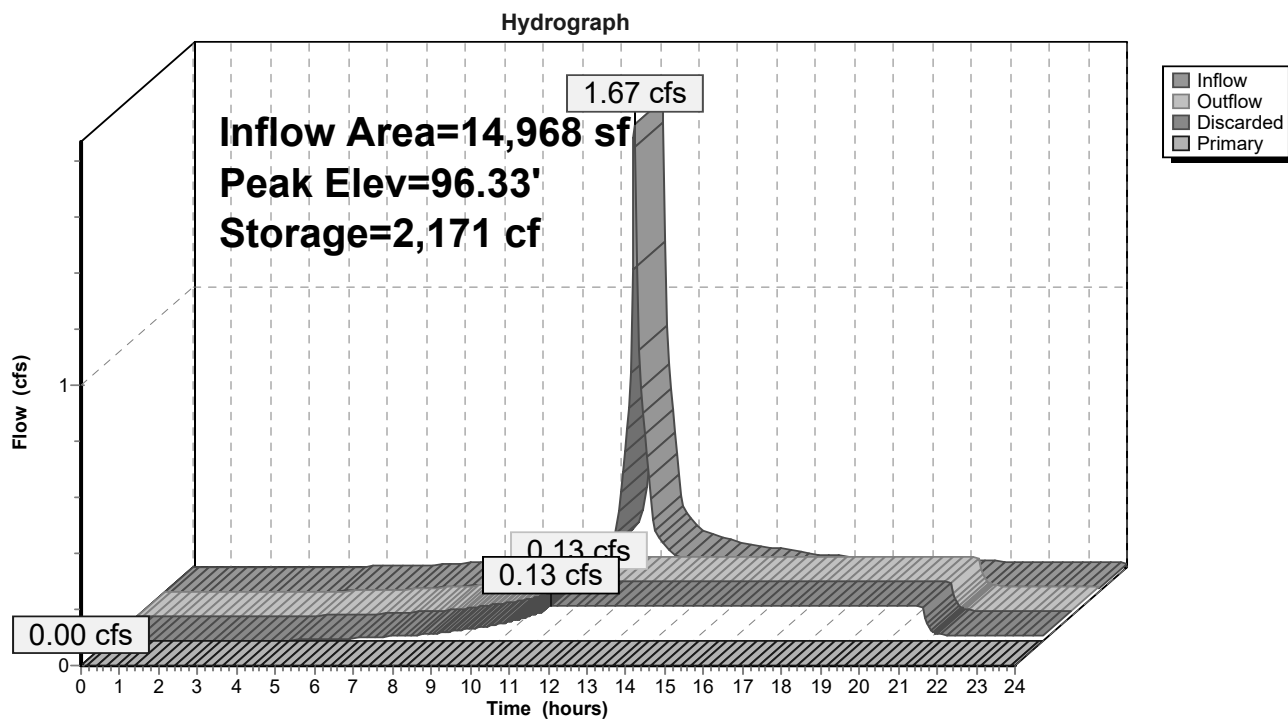
HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 10-year Rainfall=5.35"

Printed 11/18/2021

Page 24

## Pond 1P: Cultec 330XL HD





**Proposed Conditions 11-11-21**

Type III 24-hr 10-year Rainfall=5.35"

Prepared by Engineering Alliance, Inc.

Printed 11/18/2021

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Page 25

**Summary for Pond 3P: Cultec 330XL HD**

Inflow Area = 3,881 sf, 85.24% Impervious, Inflow Depth > 4.11" for 10-year event  
 Inflow = 0.42 cfs @ 12.07 hrs, Volume= 1,328 cf  
 Outflow = 0.04 cfs @ 11.50 hrs, Volume= 1,326 cf, Atten= 91%, Lag= 0.0 min  
 Discarded = 0.04 cfs @ 11.50 hrs, Volume= 1,326 cf  
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2

Peak Elev= 96.41' @ 12.99 hrs Surf.Area= 656 sf Storage= 508 cf

Plug-Flow detention time= 109.7 min calculated for 1,324 cf (100% of inflow)

Center-of-Mass det. time= 108.8 min ( 899.5 - 790.8 )

Volume	Invert	Avail.Storage	Storage Description
#1A	95.21'	578 cf	<b>20.83'W x 31.50'L x 3.54'H Field A</b> 2,324 cf Overall - 879 cf Embedded = 1,445 cf x 40.0% Voids
#2A	95.71'	879 cf	<b>Cultec R-330XLHD x 16 Inside #1</b> Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 4 rows
#3	98.80'	0 cf	<b>0.50'D x 1.08'H Vertical Cone/Cylinder-Impervious</b>
		1,457 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	95.21'	<b>2.410 in/hr Exfiltration over Surface area</b>
#2	Primary	99.83'	<b>6.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Discarded OutFlow** Max=0.04 cfs @ 11.50 hrs HW=95.26' (Free Discharge)↑**1=Exfiltration** (Exfiltration Controls 0.04 cfs)**Primary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=95.21' (Free Discharge)↑**2=Orifice/Grate** ( Controls 0.00 cfs)

## Proposed Conditions 11-11-21

Prepared by Engineering Alliance, Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 10-year Rainfall=5.35"

Printed 11/18/2021

Page 26

### Pond 3P: Cultec 330XL HD - Chamber Wizard Field A

#### Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf

Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap

Row Length Adjustment= +1.50' x 7.45 sf x 4 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

4 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 29.50' Row Length +12.0" End Stone x 2 = 31.50' Base Length

4 Rows x 52.0" Wide + 6.0" Spacing x 3 + 12.0" Side Stone x 2 = 20.83' Base Width

6.0" Stone Base + 30.5" Chamber Height + 6.0" Stone Cover = 3.54' Field Height

16 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 4 Rows = 879.2 cf Chamber Storage

2,324.2 cf Field - 879.2 cf Chambers = 1,445.0 cf Stone x 40.0% Voids = 578.0 cf Stone Storage

Chamber Storage + Stone Storage = 1,457.2 cf = 0.033 af

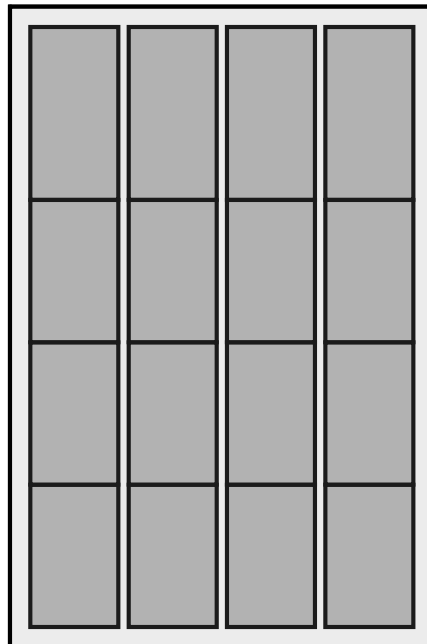
Overall Storage Efficiency = 62.7%

Overall System Size = 31.50' x 20.83' x 3.54'

16 Chambers

86.1 cy Field

53.5 cy Stone



# Proposed Conditions 11-11-21

Prepared by Engineering Alliance, Inc.

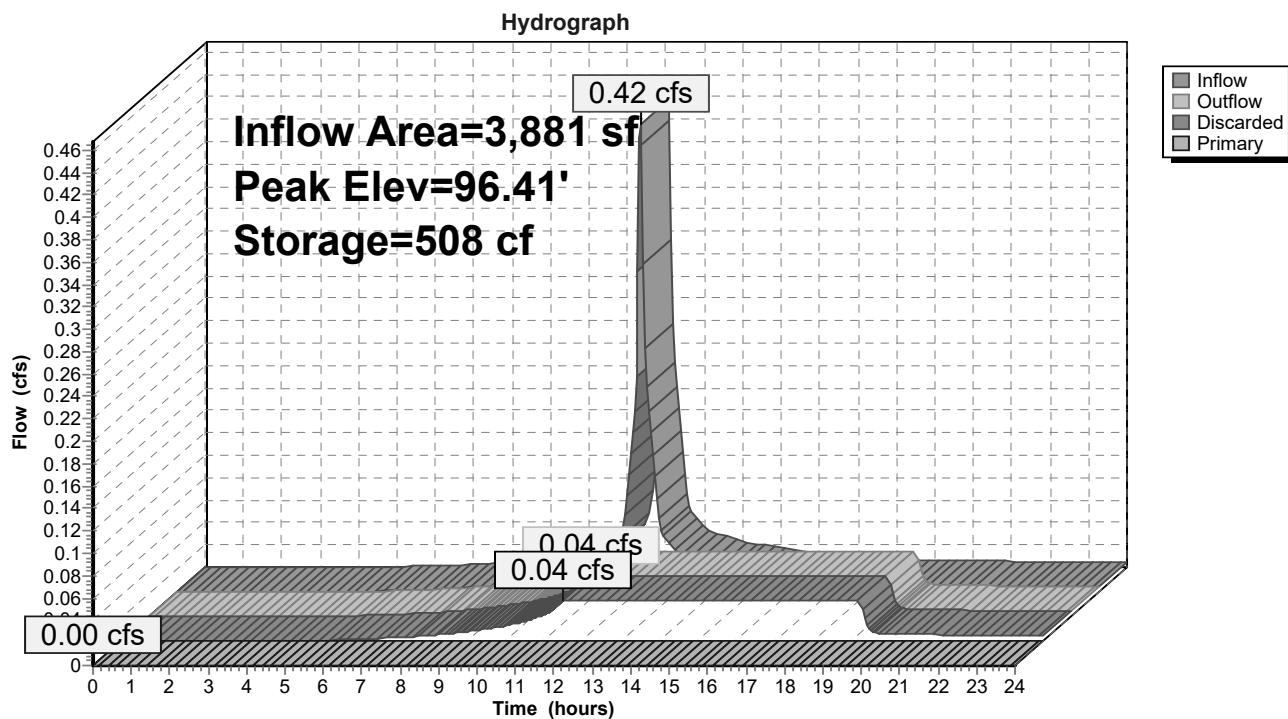
HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 10-year Rainfall=5.35"

Printed 11/18/2021

Page 27

## Pond 3P: Cultec 330XL HD



**Proposed Conditions 11-11-21***Type III 24-hr 25-year Rainfall=6.58"*

Prepared by Engineering Alliance, Inc.

Printed 11/18/2021

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Page 28

Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points  
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

<b>Subcatchment PWS-1: PWS-1</b>	Runoff Area=14,968 sf 88.88% Impervious Runoff Depth>5.52" Tc=5.0 min CN=91 Runoff=2.11 cfs 6,889 cf
<b>Subcatchment PWS-2: PWS-2</b>	Runoff Area=3,444 sf 10.80% Impervious Runoff Depth>1.04" Tc=5.0 min CN=45 Runoff=0.07 cfs 300 cf
<b>Subcatchment PWS-3: PWS-3</b>	Runoff Area=3,881 sf 85.24% Impervious Runoff Depth>5.30" Tc=5.0 min CN=89 Runoff=0.53 cfs 1,713 cf
<b>Subcatchment PWS-4: PWS-4</b>	Runoff Area=1,367 sf 0.00% Impervious Runoff Depth>0.62" Tc=5.0 min CN=39 Runoff=0.01 cfs 71 cf
<b>Reach DP-1: Hillside Ave (east)</b>	Inflow=0.07 cfs 300 cf Outflow=0.07 cfs 300 cf
<b>Reach DP-2: Allenclair Drive</b>	Inflow=0.01 cfs 71 cf Outflow=0.01 cfs 71 cf
<b>Pond 1P: Cultec 330XL HD</b>	Peak Elev=96.80' Storage=3,018 cf Inflow=2.11 cfs 6,889 cf Discarded=0.13 cfs 6,845 cf Primary=0.00 cfs 0 cf Outflow=0.13 cfs 6,845 cf
<b>Pond 3P: Cultec 330XL HD</b>	Peak Elev=96.81' Storage=712 cf Inflow=0.53 cfs 1,713 cf Discarded=0.04 cfs 1,711 cf Primary=0.00 cfs 0 cf Outflow=0.04 cfs 1,711 cf
<b>Total Runoff Area = 23,660 sf Runoff Volume = 8,972 cf Average Runoff Depth = 4.55"</b> <b>28.22% Pervious = 6,676 sf 71.78% Impervious = 16,984 sf</b>	

**Proposed Conditions 11-11-21**

Prepared by Engineering Alliance, Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 25-year Rainfall=6.58"

Printed 11/18/2021

Page 29

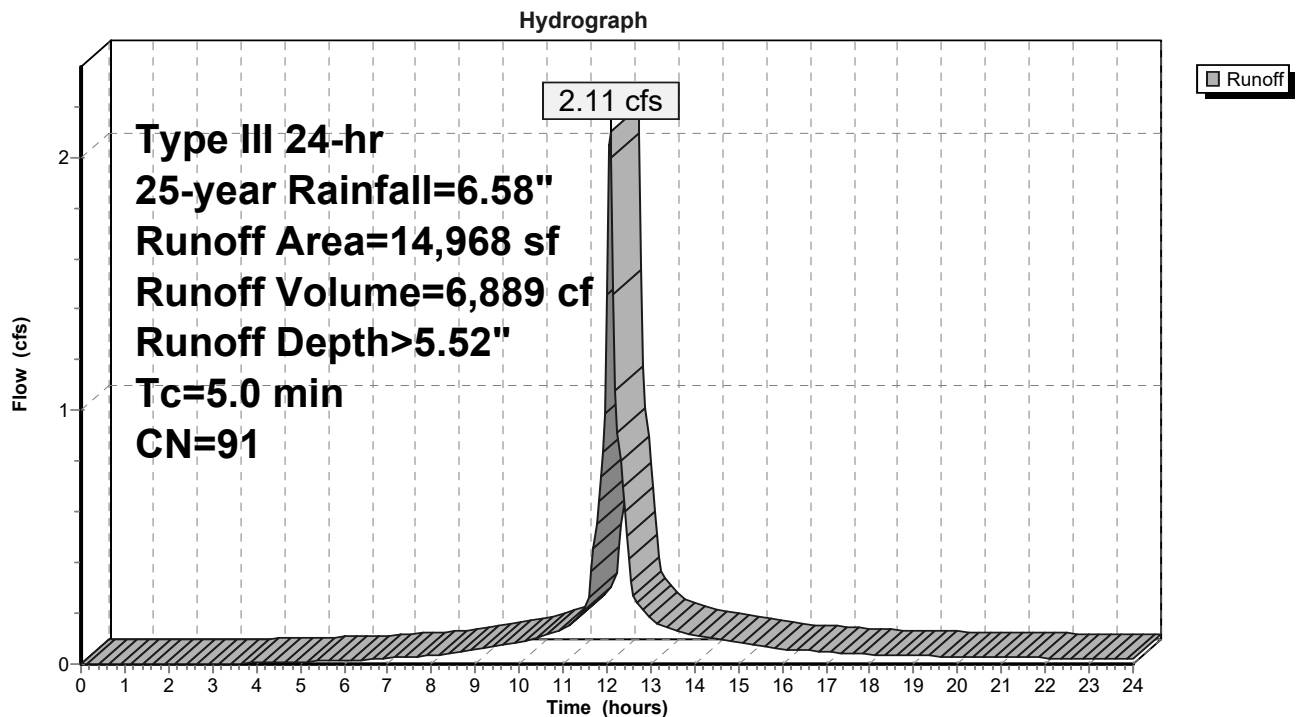
**Summary for Subcatchment PWS-1: PWS-1**

Runoff = 2.11 cfs @ 12.07 hrs, Volume= 6,889 cf, Depth&gt; 5.52"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
Type III 24-hr 25-year Rainfall=6.58"

Area (sf)	CN	Description
8,635	98	Paved parking, HSG A
4,669	98	Roofs, HSG A
1,664	39	>75% Grass cover, Good, HSG A
14,968	91	Weighted Average
1,664		11.12% Pervious Area
13,304		88.88% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

**Subcatchment PWS-1: PWS-1**

**Proposed Conditions 11-11-21**

Prepared by Engineering Alliance, Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 25-year Rainfall=6.58"

Printed 11/18/2021

Page 30

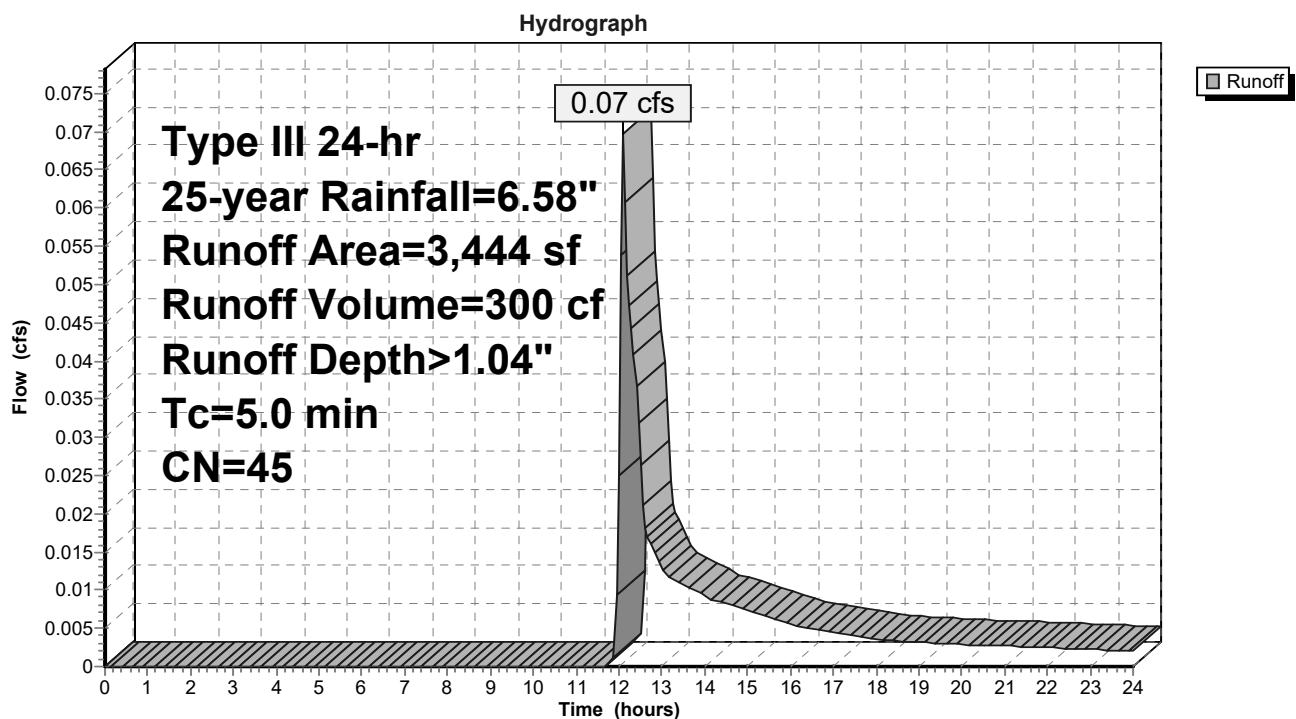
**Summary for Subcatchment PWS-2: PWS-2**

Runoff = 0.07 cfs @ 12.11 hrs, Volume= 300 cf, Depth&gt; 1.04"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
Type III 24-hr 25-year Rainfall=6.58"

Area (sf)	CN	Description
3,072	39	>75% Grass cover, Good, HSG A
372	98	Paved roads w/curbs & sewers, HSG A
3,444	45	Weighted Average
3,072		89.20% Pervious Area
372		10.80% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

**Subcatchment PWS-2: PWS-2**

**Proposed Conditions 11-11-21**

Prepared by Engineering Alliance, Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 25-year Rainfall=6.58"

Printed 11/18/2021

Page 31

**Summary for Subcatchment PWS-3: PWS-3**

Runoff = 0.53 cfs @ 12.07 hrs, Volume= 1,713 cf, Depth&gt; 5.30"

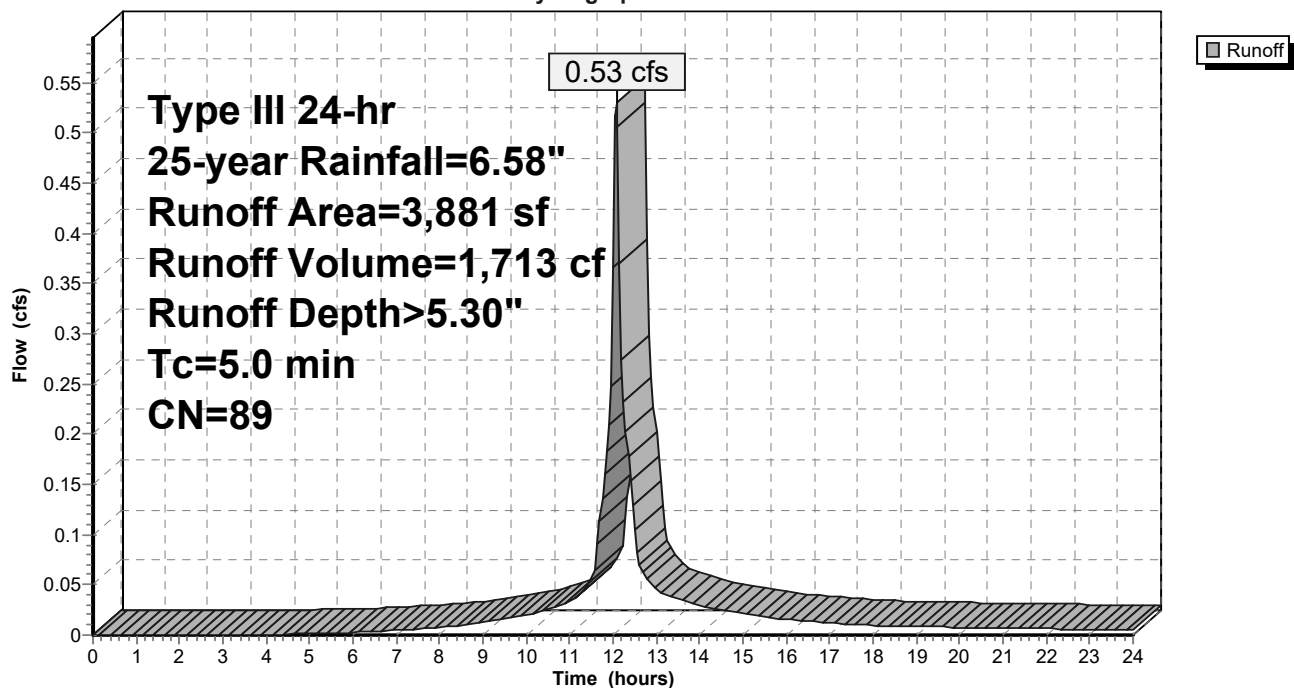
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
Type III 24-hr 25-year Rainfall=6.58"

Area (sf)	CN	Description
573	39	>75% Grass cover, Good, HSG A
3,290	98	Paved parking, HSG A
* 18	98	Walk, HSG A
3,881	89	Weighted Average
573		14.76% Pervious Area
3,308		85.24% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

**Subcatchment PWS-3: PWS-3**

Hydrograph



**Proposed Conditions 11-11-21**

Prepared by Engineering Alliance, Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 25-year Rainfall=6.58"

Printed 11/18/2021

Page 32

**Summary for Subcatchment PWS-4: PWS-4**

Runoff = 0.01 cfs @ 12.27 hrs, Volume= 71 cf, Depth&gt; 0.62"

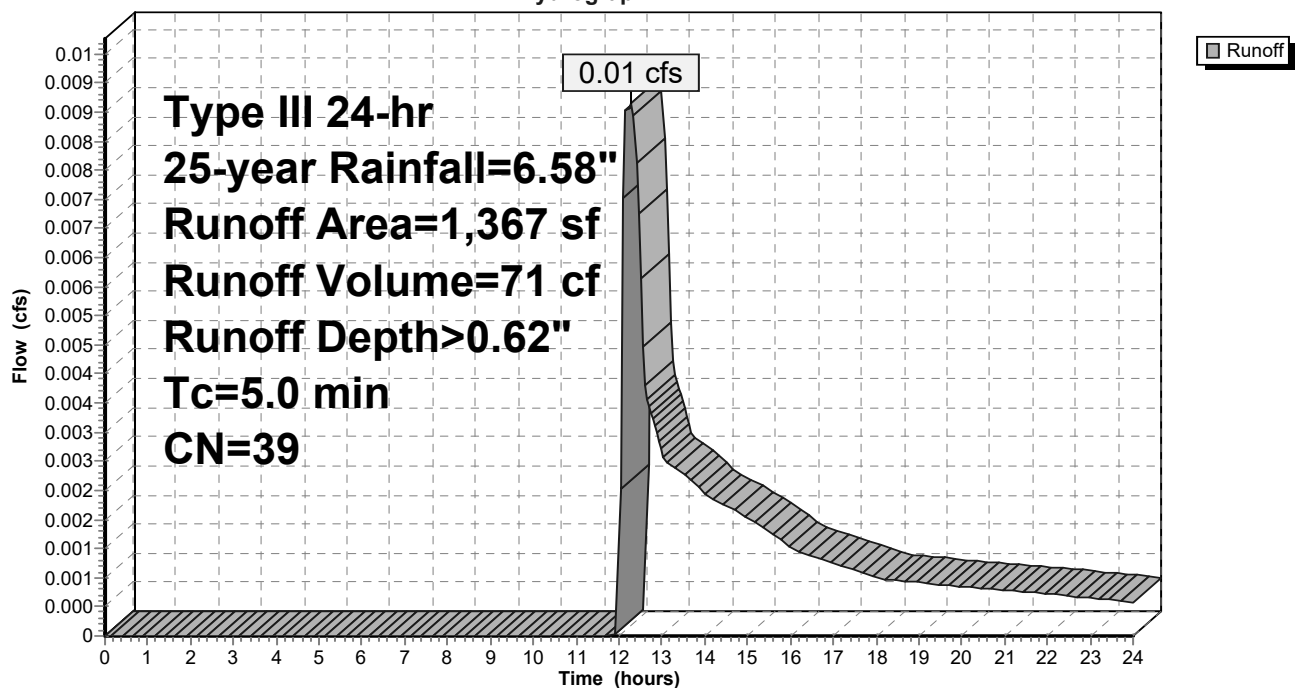
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
Type III 24-hr 25-year Rainfall=6.58"

Area (sf)	CN	Description
1,367	39	>75% Grass cover, Good, HSG A
1,367		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

**Subcatchment PWS-4: PWS-4**

Hydrograph





## Proposed Conditions 11-11-21

Prepared by Engineering Alliance, Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 25-year Rainfall=6.58"

Printed 11/18/2021

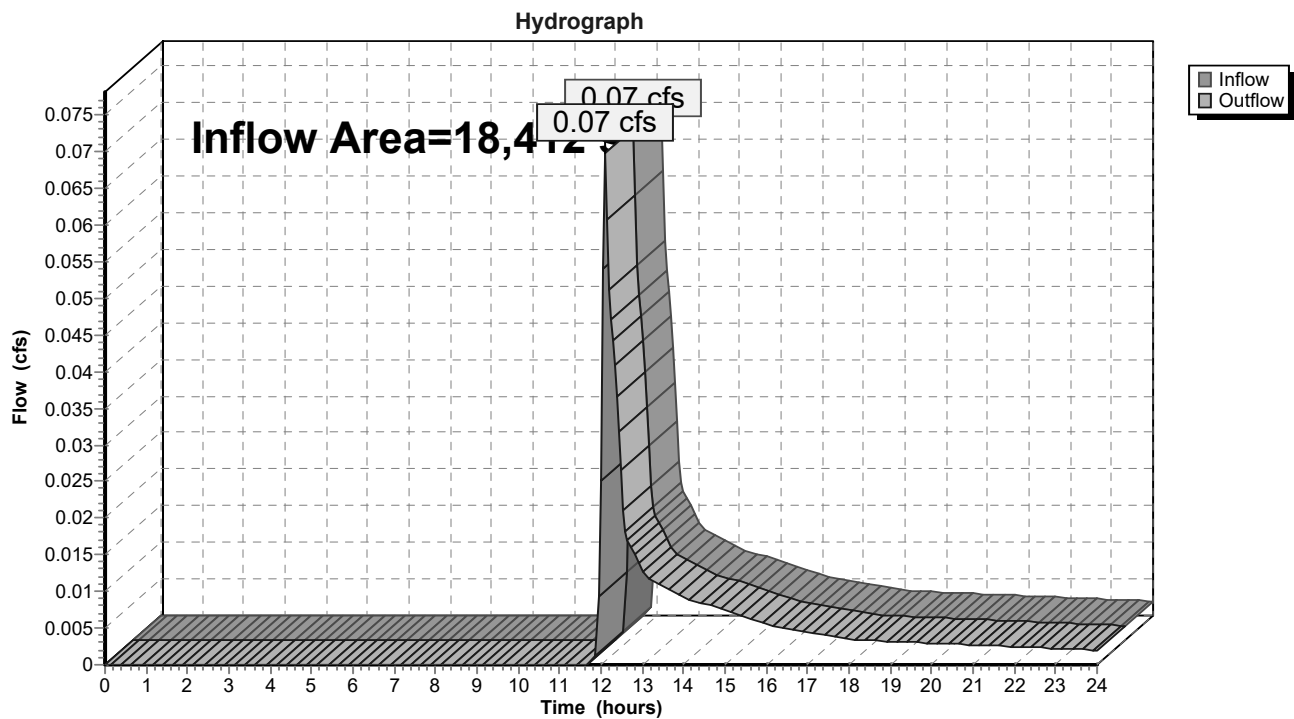
Page 33

### Summary for Reach DP-1: Hillside Ave (east)

Inflow Area = 18,412 sf, 74.28% Impervious, Inflow Depth > 0.20" for 25-year event  
Inflow = 0.07 cfs @ 12.11 hrs, Volume= 300 cf  
Outflow = 0.07 cfs @ 12.11 hrs, Volume= 300 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

### Reach DP-1: Hillside Ave (east)



## Proposed Conditions 11-11-21

Prepared by Engineering Alliance, Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 25-year Rainfall=6.58"

Printed 11/18/2021

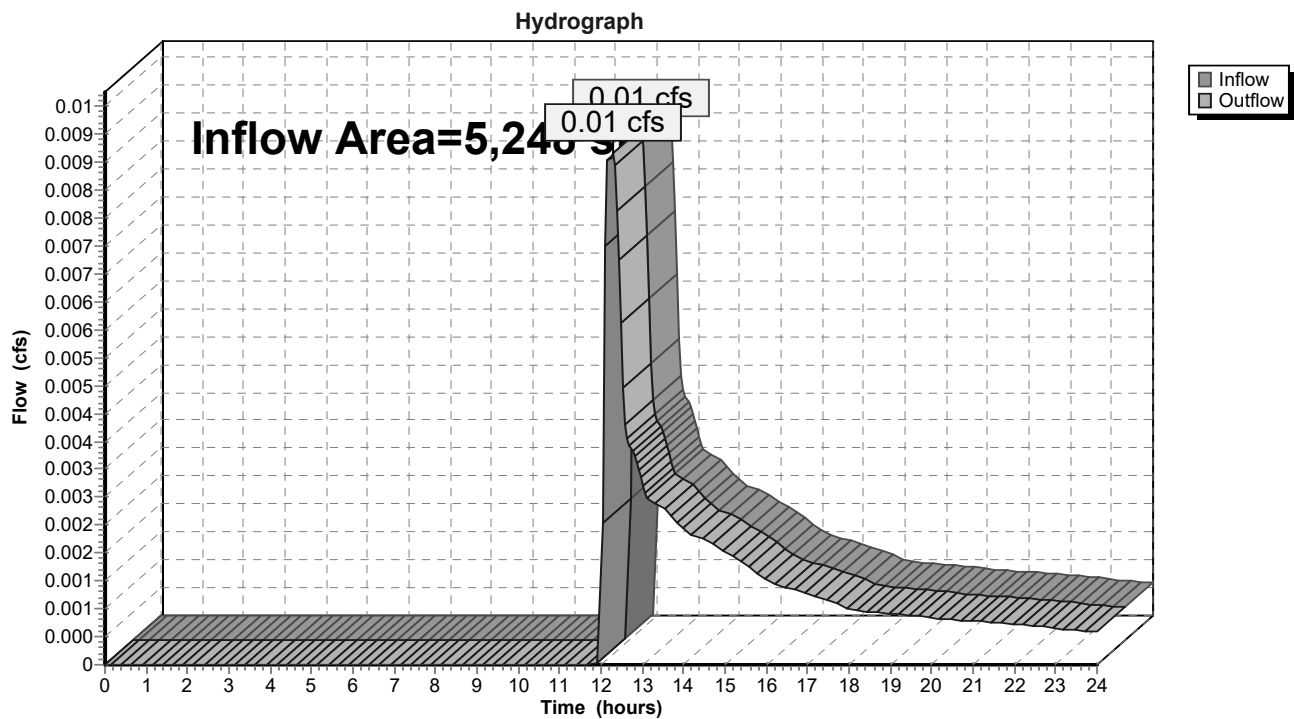
Page 34

### Summary for Reach DP-2: Allenclair Drive

Inflow Area = 5,248 sf, 63.03% Impervious, Inflow Depth > 0.16" for 25-year event  
Inflow = 0.01 cfs @ 12.27 hrs, Volume= 71 cf  
Outflow = 0.01 cfs @ 12.27 hrs, Volume= 71 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

### Reach DP-2: Allenclair Drive



**Proposed Conditions 11-11-21**

Type III 24-hr 25-year Rainfall=6.58"

Prepared by Engineering Alliance, Inc.

Printed 11/18/2021

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Page 35

**Summary for Pond 1P: Cultec 330XL HD**

Inflow Area = 14,968 sf, 88.88% Impervious, Inflow Depth > 5.52" for 25-year event  
 Inflow = 2.11 cfs @ 12.07 hrs, Volume= 6,889 cf  
 Outflow = 0.13 cfs @ 10.95 hrs, Volume= 6,845 cf, Atten= 94%, Lag= 0.0 min  
 Discarded = 0.13 cfs @ 10.95 hrs, Volume= 6,845 cf  
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2  
 Peak Elev= 96.80' @ 13.77 hrs Surf.Area= 2,242 sf Storage= 3,018 cf

Plug-Flow detention time= 205.7 min calculated for 6,831 cf (99% of inflow)  
 Center-of-Mass det. time= 201.2 min ( 978.2 - 777.0 )

Volume	Invert	Avail.Storage	Storage Description
#1A	94.91'	1,897 cf	<b>30.50'W x 73.50'L x 3.54'H Field A</b> 7,940 cf Overall - 3,196 cf Embedded = 4,743 cf x 40.0% Voids
#2A	95.41'	3,196 cf	<b>Cultec R-330XLHD x 60 Inside #1</b> Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 6 rows
#3	98.45'	0 cf	<b>0.50'D x 1.77'H Vertical Cone/Cylinder-Impervious</b>
		5,094 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	94.91'	<b>2.410 in/hr Exfiltration over Surface area</b>
#2	Primary	100.21'	<b>6.0" Vert. Orifice/Grate X 3.00</b> C= 0.600 Limited to weir flow at low heads

**Discarded OutFlow** Max=0.13 cfs @ 10.95 hrs HW=94.97' (Free Discharge)  
 ↑**1=Exfiltration** (Exfiltration Controls 0.13 cfs)

**Primary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=94.91' (Free Discharge)  
 ↑**2=Orifice/Grate** ( Controls 0.00 cfs)

## Proposed Conditions 11-11-21

Prepared by Engineering Alliance, Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 25-year Rainfall=6.58"

Printed 11/18/2021

Page 36

### Pond 1P: Cultec 330XL HD - Chamber Wizard Field A

#### Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf

Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap

Row Length Adjustment= +1.50' x 7.45 sf x 6 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

10 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 71.50' Row Length +12.0" End Stone x 2 = 73.50' Base Length

6 Rows x 52.0" Wide + 6.0" Spacing x 5 + 12.0" Side Stone x 2 = 30.50' Base Width

6.0" Stone Base + 30.5" Chamber Height + 6.0" Stone Cover = 3.54' Field Height

60 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 6 Rows = 3,196.5 cf Chamber Storage

7,939.5 cf Field - 3,196.5 cf Chambers = 4,743.1 cf Stone x 40.0% Voids = 1,897.2 cf Stone Storage

Chamber Storage + Stone Storage = 5,093.7 cf = 0.117 af

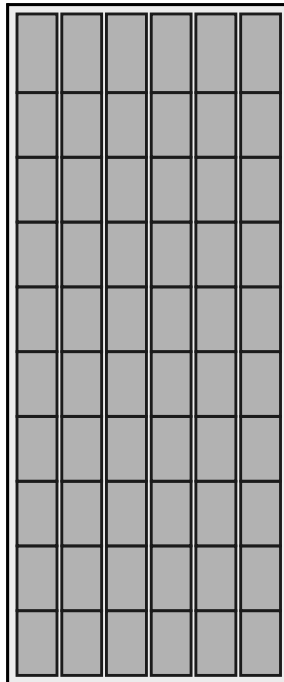
Overall Storage Efficiency = 64.2%

Overall System Size = 73.50' x 30.50' x 3.54'

60 Chambers

294.1 cy Field

175.7 cy Stone



# Proposed Conditions 11-11-21

Prepared by Engineering Alliance, Inc.

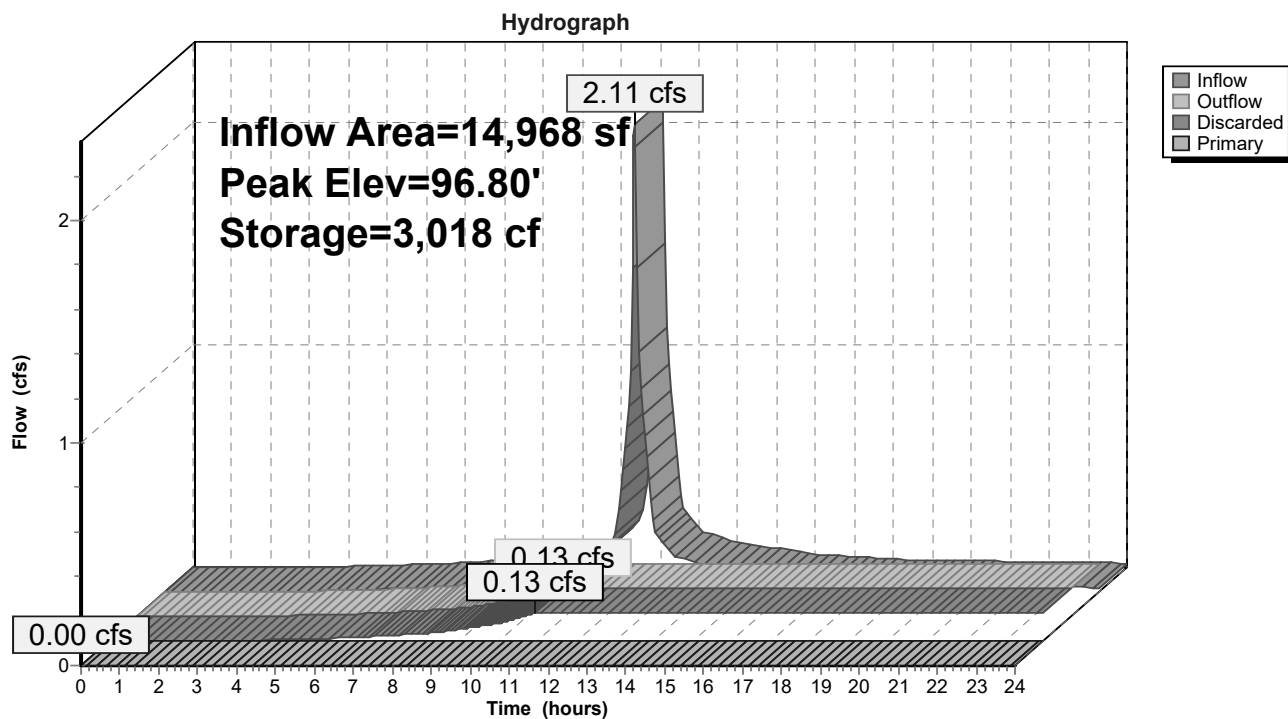
HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 25-year Rainfall=6.58"

Printed 11/18/2021

Page 37

## Pond 1P: Cultec 330XL HD



**Proposed Conditions 11-11-21**

Type III 24-hr 25-year Rainfall=6.58"

Prepared by Engineering Alliance, Inc.

Printed 11/18/2021

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Page 38

**Summary for Pond 3P: Cultec 330XL HD**

Inflow Area = 3,881 sf, 85.24% Impervious, Inflow Depth > 5.30" for 25-year event  
 Inflow = 0.53 cfs @ 12.07 hrs, Volume= 1,713 cf  
 Outflow = 0.04 cfs @ 11.25 hrs, Volume= 1,711 cf, Atten= 93%, Lag= 0.0 min  
 Discarded = 0.04 cfs @ 11.25 hrs, Volume= 1,711 cf  
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2

Peak Elev= 96.81' @ 13.43 hrs Surf.Area= 656 sf Storage= 712 cf

Plug-Flow detention time= 161.9 min calculated for 1,707 cf (100% of inflow)

Center-of-Mass det. time= 160.8 min ( 944.7 - 783.9 )

Volume	Invert	Avail.Storage	Storage Description
#1A	95.21'	578 cf	<b>20.83'W x 31.50'L x 3.54'H Field A</b> 2,324 cf Overall - 879 cf Embedded = 1,445 cf x 40.0% Voids
#2A	95.71'	879 cf	<b>Cultec R-330XLHD x 16 Inside #1</b> Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 4 rows
#3	98.80'	0 cf	<b>0.50'D x 1.08'H Vertical Cone/Cylinder-Impervious</b>
		1,457 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	95.21'	<b>2.410 in/hr Exfiltration over Surface area</b>
#2	Primary	99.83'	<b>6.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Discarded OutFlow** Max=0.04 cfs @ 11.25 hrs HW=95.26' (Free Discharge)↑**1=Exfiltration** (Exfiltration Controls 0.04 cfs)**Primary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=95.21' (Free Discharge)↑**2=Orifice/Grate** ( Controls 0.00 cfs)

## Proposed Conditions 11-11-21

Prepared by Engineering Alliance, Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 25-year Rainfall=6.58"

Printed 11/18/2021

Page 39

### Pond 3P: Cultec 330XL HD - Chamber Wizard Field A

#### Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf

Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap

Row Length Adjustment= +1.50' x 7.45 sf x 4 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

4 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 29.50' Row Length +12.0" End Stone x 2 = 31.50' Base Length

4 Rows x 52.0" Wide + 6.0" Spacing x 3 + 12.0" Side Stone x 2 = 20.83' Base Width

6.0" Stone Base + 30.5" Chamber Height + 6.0" Stone Cover = 3.54' Field Height

16 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 4 Rows = 879.2 cf Chamber Storage

2,324.2 cf Field - 879.2 cf Chambers = 1,445.0 cf Stone x 40.0% Voids = 578.0 cf Stone Storage

Chamber Storage + Stone Storage = 1,457.2 cf = 0.033 af

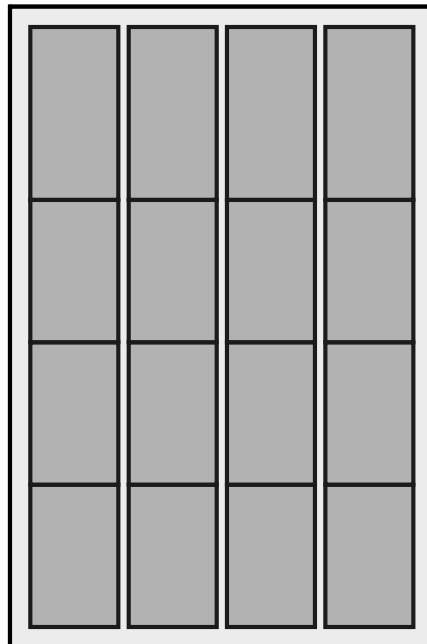
Overall Storage Efficiency = 62.7%

Overall System Size = 31.50' x 20.83' x 3.54'

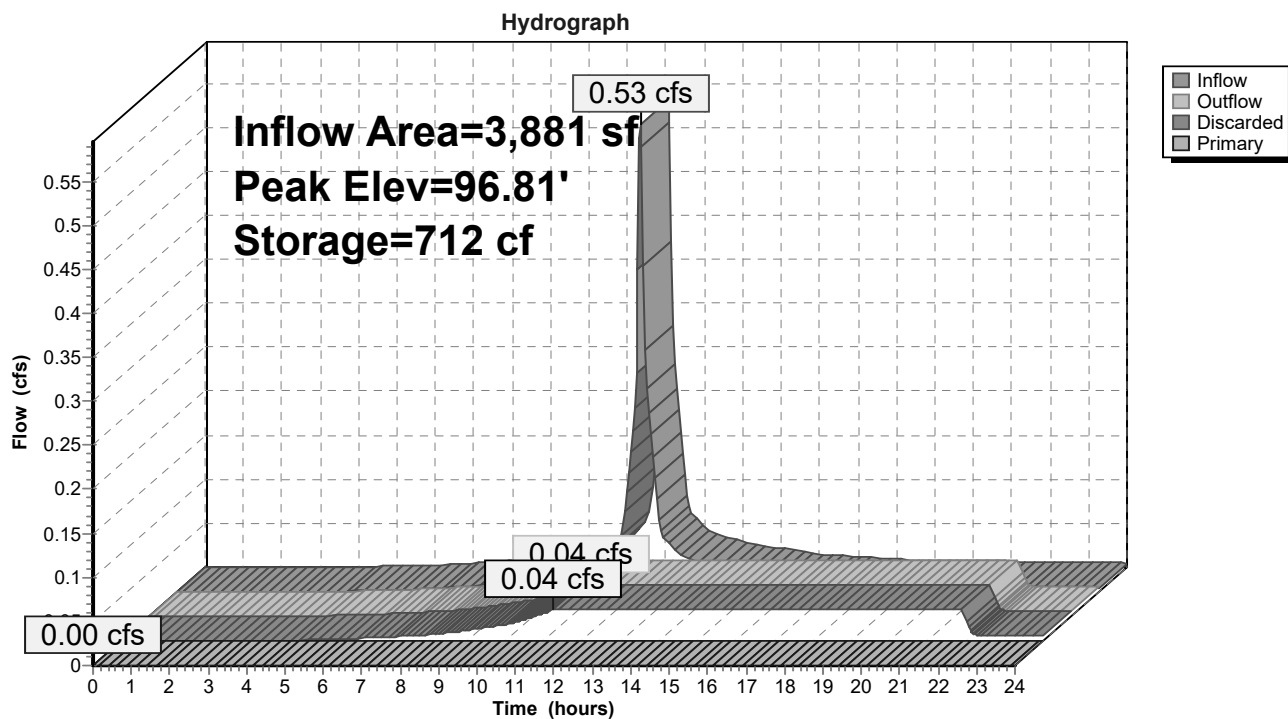
16 Chambers

86.1 cy Field

53.5 cy Stone



Pond 3P: Cultec 330XL HD





**Proposed Conditions 11-11-21***Type III 24-hr 100-year Rainfall=8.47"*

Prepared by Engineering Alliance, Inc.

Printed 11/18/2021

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Page 41

Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points  
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

<b>Subcatchment PWS-1: PWS-1</b>	Runoff Area=14,968 sf 88.88% Impervious Runoff Depth>7.39" Tc=5.0 min CN=91 Runoff=2.77 cfs 9,212 cf
<b>Subcatchment PWS-2: PWS-2</b>	Runoff Area=3,444 sf 10.80% Impervious Runoff Depth>1.99" Tc=5.0 min CN=45 Runoff=0.16 cfs 570 cf
<b>Subcatchment PWS-3: PWS-3</b>	Runoff Area=3,881 sf 85.24% Impervious Runoff Depth>7.14" Tc=5.0 min CN=89 Runoff=0.70 cfs 2,311 cf
<b>Subcatchment PWS-4: PWS-4</b>	Runoff Area=1,367 sf 0.00% Impervious Runoff Depth>1.36" Tc=5.0 min CN=39 Runoff=0.04 cfs 155 cf
<b>Reach DP-1: Hillside Ave (east)</b>	Inflow=0.16 cfs 570 cf Outflow=0.16 cfs 570 cf
<b>Reach DP-2: Allenclair Drive</b>	Inflow=0.04 cfs 155 cf Outflow=0.04 cfs 155 cf
<b>Pond 1P: Cultec 330XL HD</b>	Peak Elev=97.79' Storage=4,486 cf Inflow=2.77 cfs 9,212 cf Discarded=0.13 cfs 7,255 cf Primary=0.00 cfs 0 cf Outflow=0.13 cfs 7,255 cf
<b>Pond 3P: Cultec 330XL HD</b>	Peak Elev=97.54' Storage=1,068 cf Inflow=0.70 cfs 2,311 cf Discarded=0.04 cfs 2,048 cf Primary=0.00 cfs 0 cf Outflow=0.04 cfs 2,048 cf
<b>Total Runoff Area = 23,660 sf Runoff Volume = 12,247 cf Average Runoff Depth = 6.21"</b> <b>28.22% Pervious = 6,676 sf 71.78% Impervious = 16,984 sf</b>	

**Proposed Conditions 11-11-21**

Prepared by Engineering Alliance, Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 100-year Rainfall=8.47"

Printed 11/18/2021

Page 42

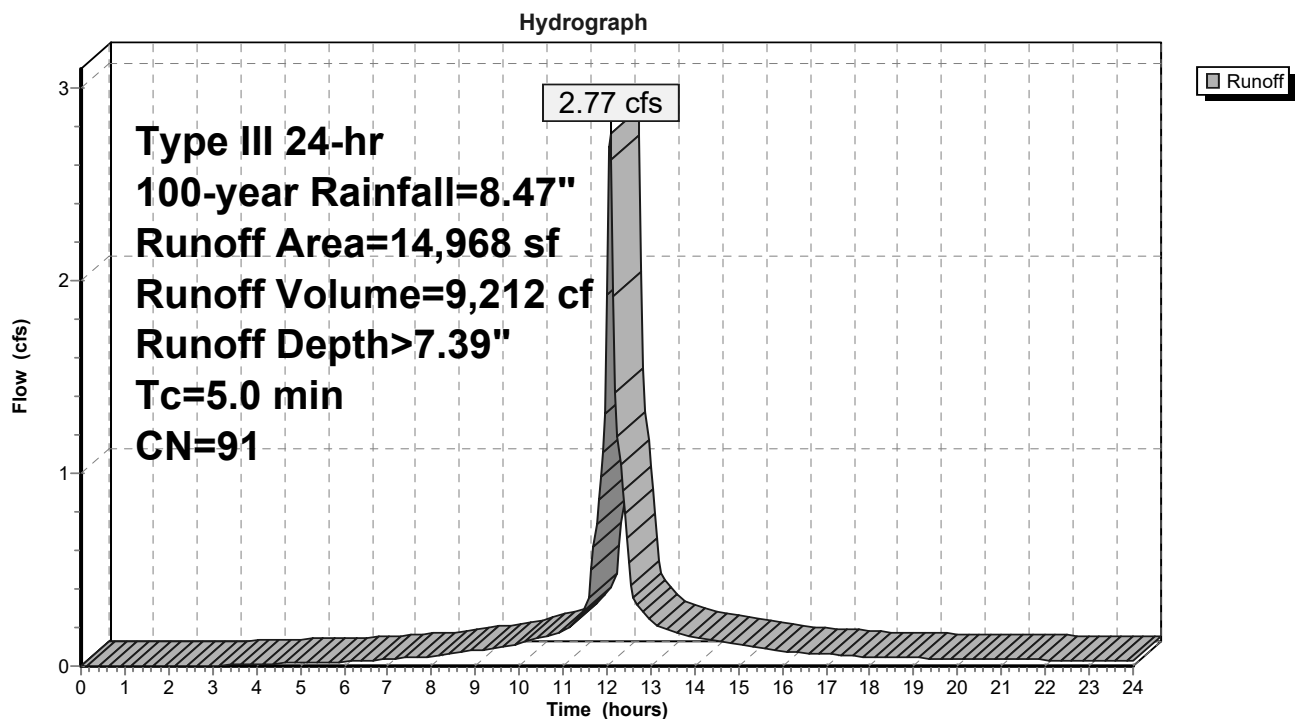
**Summary for Subcatchment PWS-1: PWS-1**

Runoff = 2.77 cfs @ 12.07 hrs, Volume= 9,212 cf, Depth&gt; 7.39"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
Type III 24-hr 100-year Rainfall=8.47"

Area (sf)	CN	Description
8,635	98	Paved parking, HSG A
4,669	98	Roofs, HSG A
1,664	39	>75% Grass cover, Good, HSG A
14,968	91	Weighted Average
1,664		11.12% Pervious Area
13,304		88.88% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

**Subcatchment PWS-1: PWS-1**

**Proposed Conditions 11-11-21**

Prepared by Engineering Alliance, Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 100-year Rainfall=8.47"

Printed 11/18/2021

Page 43

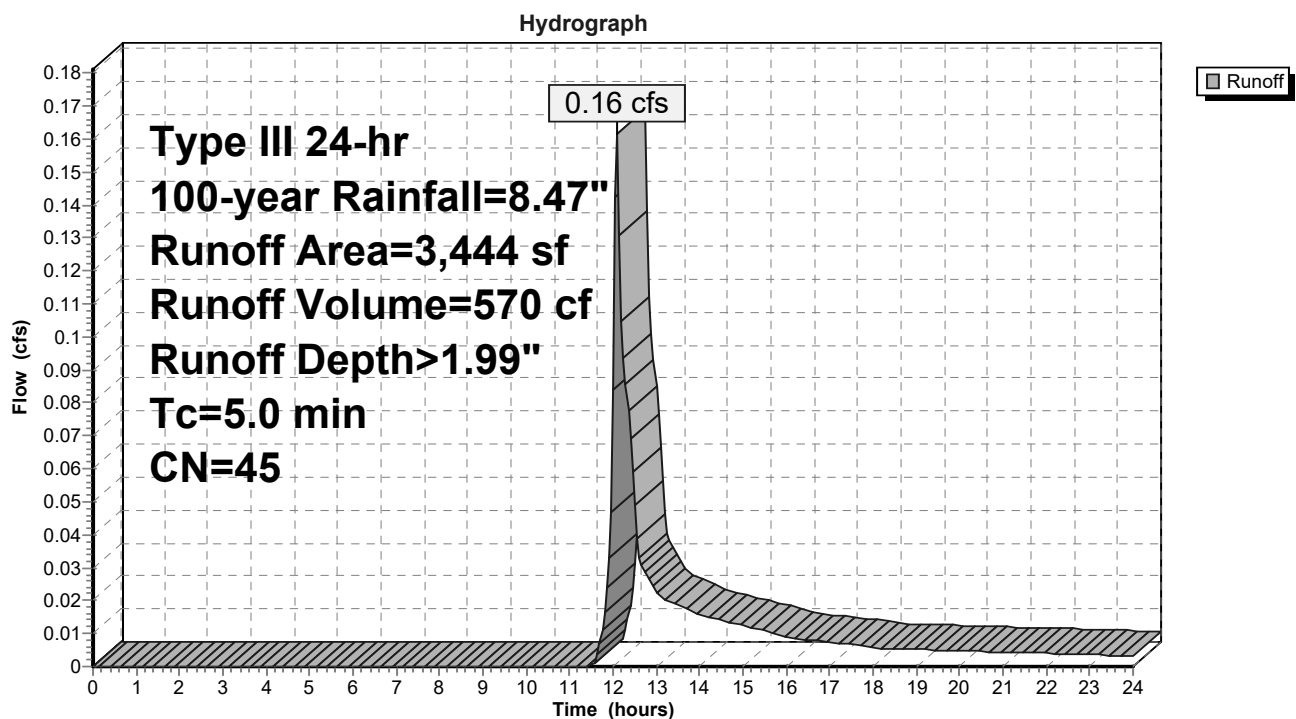
**Summary for Subcatchment PWS-2: PWS-2**

Runoff = 0.16 cfs @ 12.09 hrs, Volume= 570 cf, Depth&gt; 1.99"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
Type III 24-hr 100-year Rainfall=8.47"

Area (sf)	CN	Description
3,072	39	>75% Grass cover, Good, HSG A
372	98	Paved roads w/curbs & sewers, HSG A
3,444	45	Weighted Average
3,072		89.20% Pervious Area
372		10.80% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

**Subcatchment PWS-2: PWS-2**

**Proposed Conditions 11-11-21**

Prepared by Engineering Alliance, Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 100-year Rainfall=8.47"

Printed 11/18/2021

Page 44

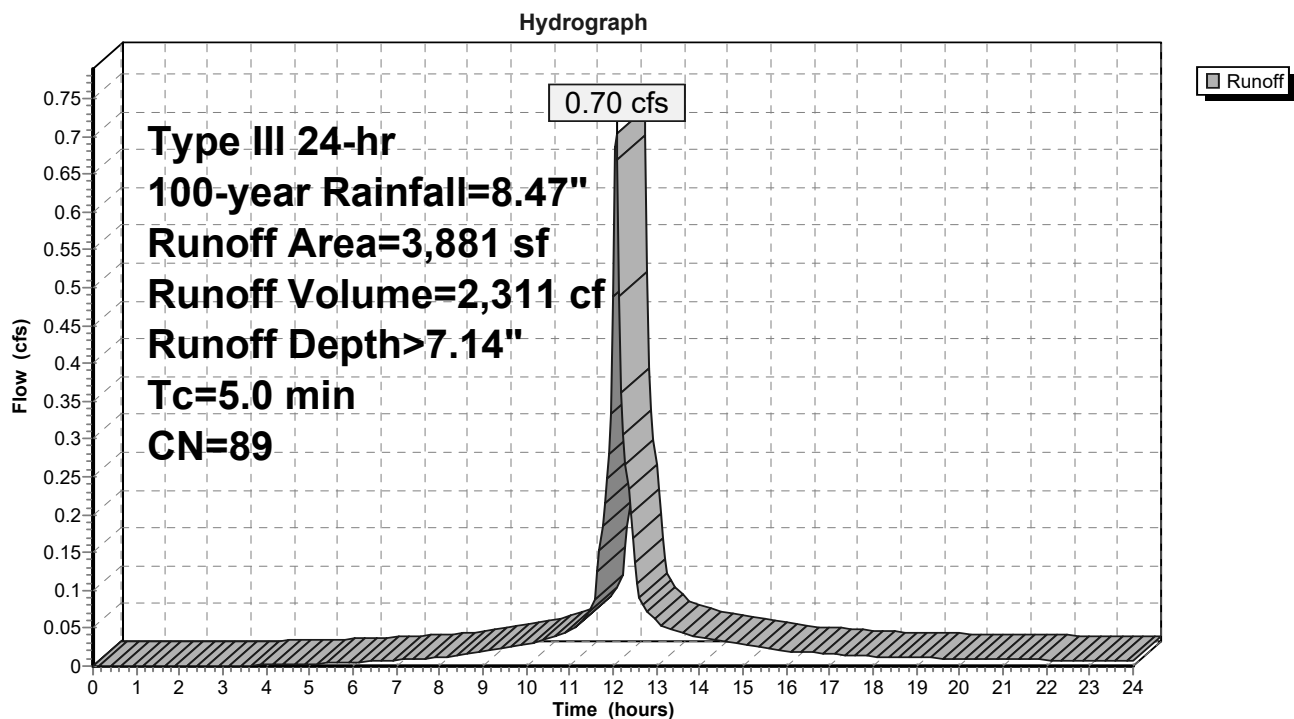
**Summary for Subcatchment PWS-3: PWS-3**

Runoff = 0.70 cfs @ 12.07 hrs, Volume= 2,311 cf, Depth&gt; 7.14"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
Type III 24-hr 100-year Rainfall=8.47"

Area (sf)	CN	Description
573	39	>75% Grass cover, Good, HSG A
3,290	98	Paved parking, HSG A
* 18	98	Walk, HSG A
3,881	89	Weighted Average
573		14.76% Pervious Area
3,308		85.24% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

**Subcatchment PWS-3: PWS-3**

**Proposed Conditions 11-11-21**

Prepared by Engineering Alliance, Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 100-year Rainfall=8.47"

Printed 11/18/2021

Page 45

**Summary for Subcatchment PWS-4: PWS-4**

Runoff = 0.04 cfs @ 12.11 hrs, Volume= 155 cf, Depth&gt; 1.36"

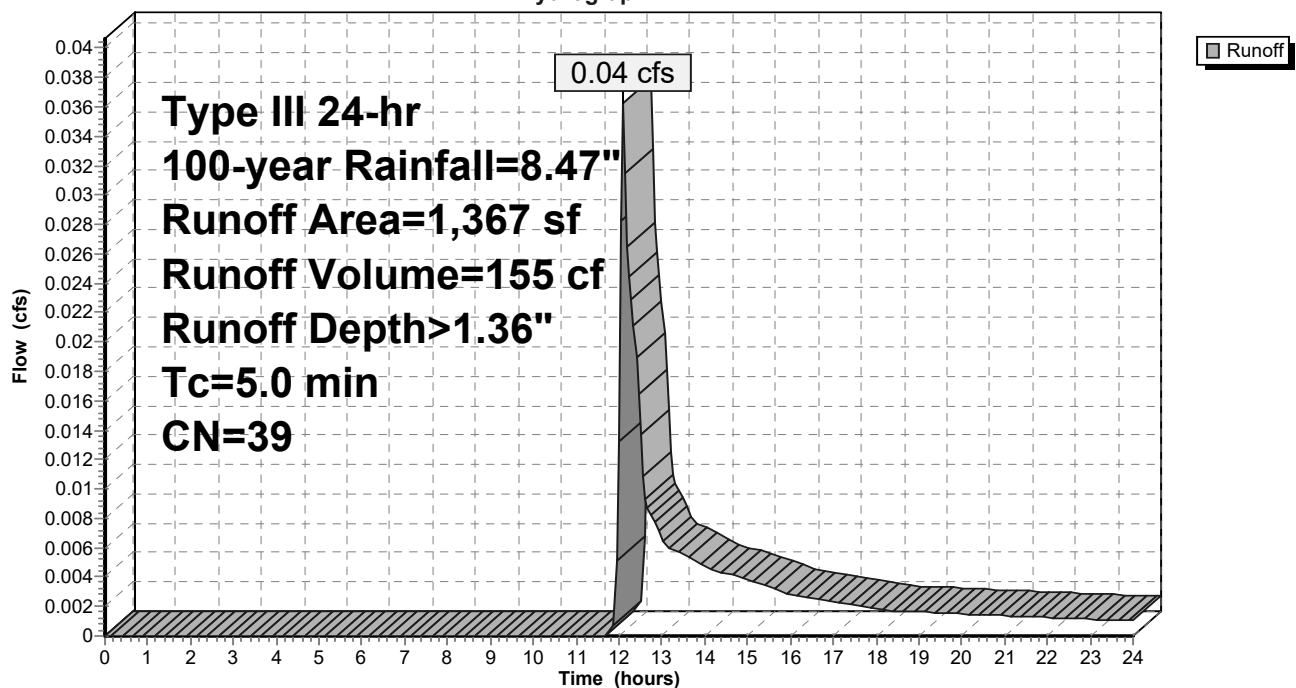
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
Type III 24-hr 100-year Rainfall=8.47"

Area (sf)	CN	Description
1,367	39	>75% Grass cover, Good, HSG A
1,367		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

**Subcatchment PWS-4: PWS-4**

Hydrograph



## Proposed Conditions 11-11-21

Prepared by Engineering Alliance, Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 100-year Rainfall=8.47"

Printed 11/18/2021

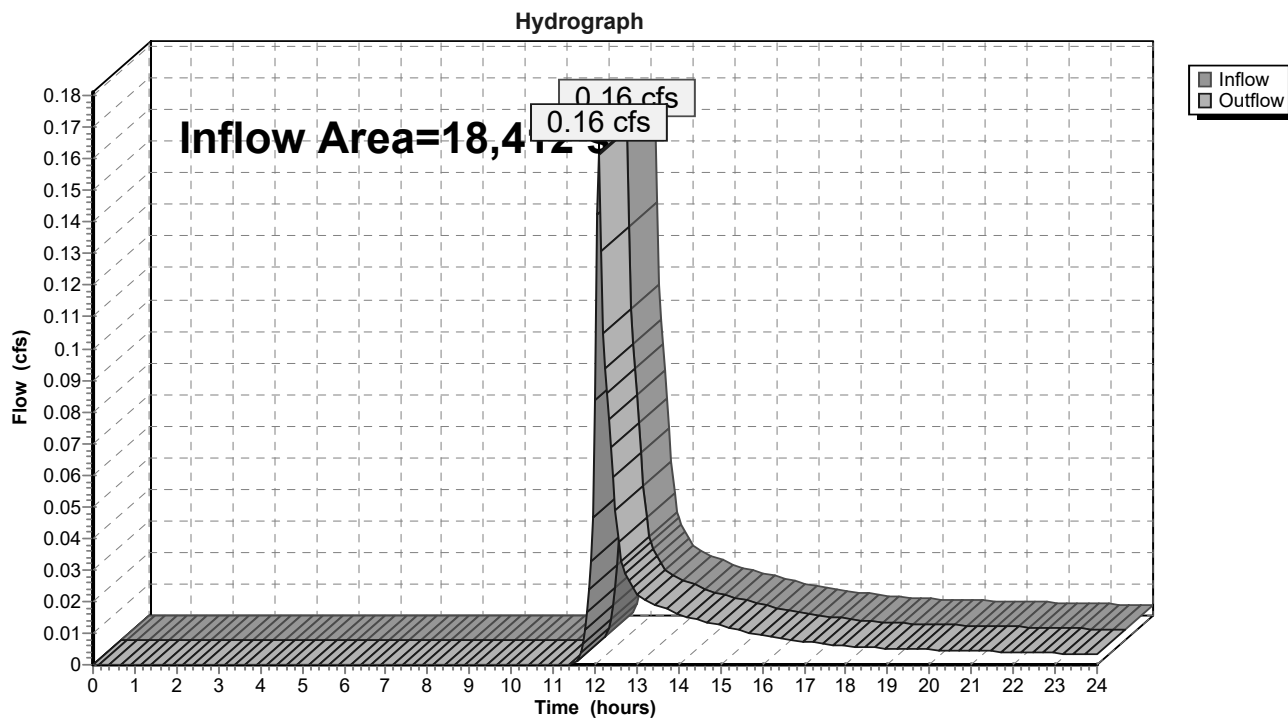
Page 46

### Summary for Reach DP-1: Hillside Ave (east)

Inflow Area = 18,412 sf, 74.28% Impervious, Inflow Depth > 0.37" for 100-year event  
Inflow = 0.16 cfs @ 12.09 hrs, Volume= 570 cf  
Outflow = 0.16 cfs @ 12.09 hrs, Volume= 570 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

### Reach DP-1: Hillside Ave (east)



## Proposed Conditions 11-11-21

Prepared by Engineering Alliance, Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 100-year Rainfall=8.47"

Printed 11/18/2021

Page 47

### Summary for Reach DP-2: Allenclair Drive

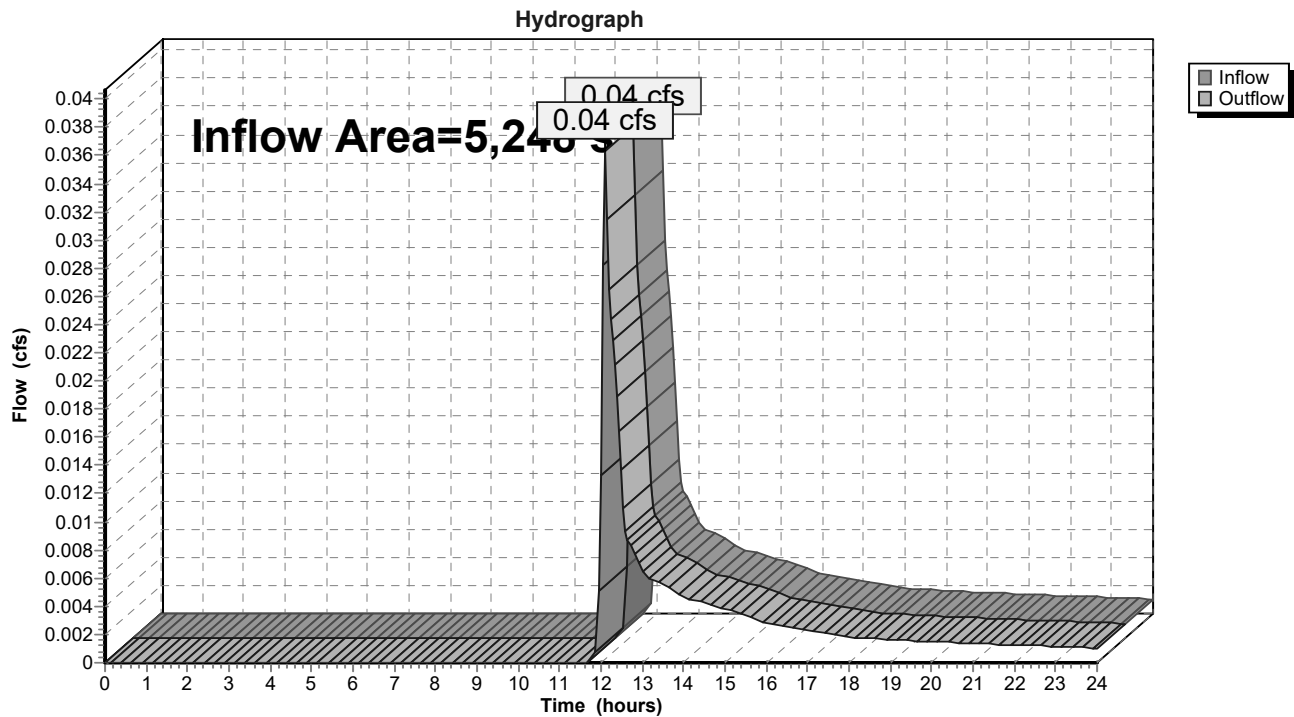
Inflow Area = 5,248 sf, 63.03% Impervious, Inflow Depth > 0.35" for 100-year event

Inflow = 0.04 cfs @ 12.11 hrs, Volume= 155 cf

Outflow = 0.04 cfs @ 12.11 hrs, Volume= 155 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

### Reach DP-2: Allenclair Drive



**Proposed Conditions 11-11-21**

Type III 24-hr 100-year Rainfall=8.47"

Prepared by Engineering Alliance, Inc.

Printed 11/18/2021

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Page 48

**Summary for Pond 1P: Cultec 330XL HD**

Inflow Area = 14,968 sf, 88.88% Impervious, Inflow Depth > 7.39" for 100-year event  
 Inflow = 2.77 cfs @ 12.07 hrs, Volume= 9,212 cf  
 Outflow = 0.13 cfs @ 10.30 hrs, Volume= 7,255 cf, Atten= 95%, Lag= 0.0 min  
 Discarded = 0.13 cfs @ 10.30 hrs, Volume= 7,255 cf  
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2  
 Peak Elev= 97.79' @ 14.57 hrs Surf.Area= 2,242 sf Storage= 4,486 cf

Plug-Flow detention time= 258.6 min calculated for 7,255 cf (79% of inflow)  
 Center-of-Mass det. time= 180.8 min ( 950.5 - 769.7 )

Volume	Invert	Avail.Storage	Storage Description
#1A	94.91'	1,897 cf	<b>30.50'W x 73.50'L x 3.54'H Field A</b> 7,940 cf Overall - 3,196 cf Embedded = 4,743 cf x 40.0% Voids
#2A	95.41'	3,196 cf	<b>Cultec R-330XLHD x 60 Inside #1</b> Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 6 rows
#3	98.45'	0 cf	<b>0.50'D x 1.77'H Vertical Cone/Cylinder-Impervious</b>
		5,094 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	94.91'	<b>2.410 in/hr Exfiltration over Surface area</b>
#2	Primary	100.21'	<b>6.0" Vert. Orifice/Grate X 3.00</b> C= 0.600 Limited to weir flow at low heads

**Discarded OutFlow** Max=0.13 cfs @ 10.30 hrs HW=94.97' (Free Discharge)  
 ↑**1=Exfiltration** (Exfiltration Controls 0.13 cfs)

**Primary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=94.91' (Free Discharge)  
 ↑**2=Orifice/Grate** ( Controls 0.00 cfs)



## Proposed Conditions 11-11-21

Prepared by Engineering Alliance, Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 100-year Rainfall=8.47"

Printed 11/18/2021

Page 49

### Pond 1P: Cultec 330XL HD - Chamber Wizard Field A

#### Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf

Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap

Row Length Adjustment= +1.50' x 7.45 sf x 6 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

10 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 71.50' Row Length +12.0" End Stone x 2 = 73.50' Base Length

6 Rows x 52.0" Wide + 6.0" Spacing x 5 + 12.0" Side Stone x 2 = 30.50' Base Width

6.0" Stone Base + 30.5" Chamber Height + 6.0" Stone Cover = 3.54' Field Height

60 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 6 Rows = 3,196.5 cf Chamber Storage

7,939.5 cf Field - 3,196.5 cf Chambers = 4,743.1 cf Stone x 40.0% Voids = 1,897.2 cf Stone Storage

Chamber Storage + Stone Storage = 5,093.7 cf = 0.117 af

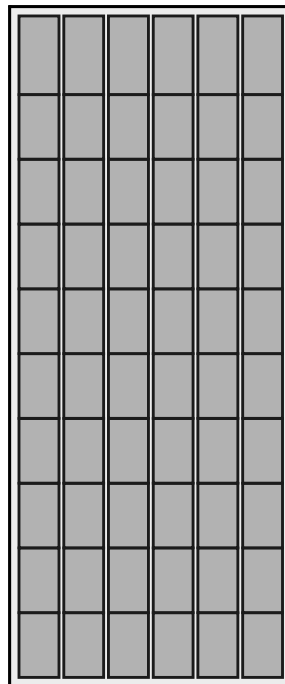
Overall Storage Efficiency = 64.2%

Overall System Size = 73.50' x 30.50' x 3.54'

60 Chambers

294.1 cy Field

175.7 cy Stone



**Proposed Conditions 11-11-21**

Prepared by Engineering Alliance, Inc.

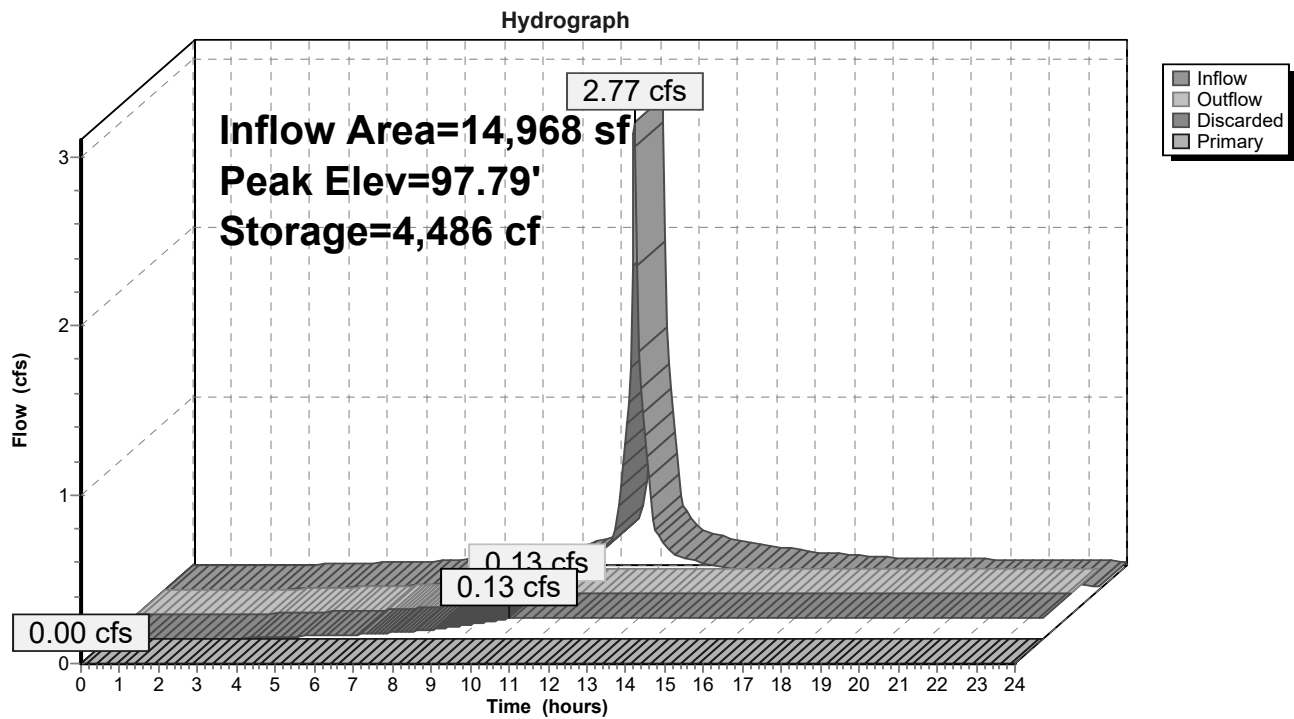
HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

*Type III 24-hr 100-year Rainfall=8.47"*

Printed 11/18/2021

Page 50

**Pond 1P: Cultec 330XL HD**



**Proposed Conditions 11-11-21**

Type III 24-hr 100-year Rainfall=8.47"

Prepared by Engineering Alliance, Inc.

Printed 11/18/2021

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Page 51

**Summary for Pond 3P: Cultec 330XL HD**

Inflow Area = 3,881 sf, 85.24% Impervious, Inflow Depth > 7.14" for 100-year event  
 Inflow = 0.70 cfs @ 12.07 hrs, Volume= 2,311 cf  
 Outflow = 0.04 cfs @ 10.65 hrs, Volume= 2,048 cf, Atten= 95%, Lag= 0.0 min  
 Discarded = 0.04 cfs @ 10.65 hrs, Volume= 2,048 cf  
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2

Peak Elev= 97.54' @ 14.07 hrs Surf.Area= 656 sf Storage= 1,068 cf

Plug-Flow detention time= 245.2 min calculated for 2,043 cf (88% of inflow)

Center-of-Mass det. time= 192.4 min ( 968.4 - 776.1 )

Volume	Invert	Avail.Storage	Storage Description
#1A	95.21'	578 cf	<b>20.83'W x 31.50'L x 3.54'H Field A</b> 2,324 cf Overall - 879 cf Embedded = 1,445 cf x 40.0% Voids
#2A	95.71'	879 cf	<b>Cultec R-330XLHD x 16 Inside #1</b> Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 4 rows
#3	98.80'	0 cf	<b>0.50'D x 1.08'H Vertical Cone/Cylinder-Impervious</b>
		1,457 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	95.21'	<b>2.410 in/hr Exfiltration over Surface area</b>
#2	Primary	99.83'	<b>6.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Discarded OutFlow** Max=0.04 cfs @ 10.65 hrs HW=95.26' (Free Discharge)↑**1=Exfiltration** (Exfiltration Controls 0.04 cfs)**Primary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=95.21' (Free Discharge)↑**2=Orifice/Grate** ( Controls 0.00 cfs)

**Proposed Conditions 11-11-21**

Prepared by Engineering Alliance, Inc.

HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 100-year Rainfall=8.47"

Printed 11/18/2021

Page 52

**Pond 3P: Cultec 330XL HD - Chamber Wizard Field A****Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)**

Effective Size= 47.8"W x 30.0"H =&gt; 7.45 sf x 7.00'L = 52.2 cf

Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap

Row Length Adjustment= +1.50' x 7.45 sf x 4 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

4 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 29.50' Row Length +12.0" End Stone x 2 = 31.50' Base Length

4 Rows x 52.0" Wide + 6.0" Spacing x 3 + 12.0" Side Stone x 2 = 20.83' Base Width

6.0" Stone Base + 30.5" Chamber Height + 6.0" Stone Cover = 3.54' Field Height

16 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 4 Rows = 879.2 cf Chamber Storage

2,324.2 cf Field - 879.2 cf Chambers = 1,445.0 cf Stone x 40.0% Voids = 578.0 cf Stone Storage

Chamber Storage + Stone Storage = 1,457.2 cf = 0.033 af

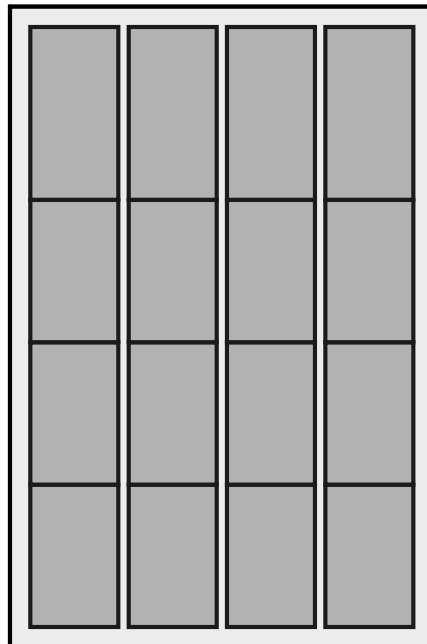
Overall Storage Efficiency = 62.7%

Overall System Size = 31.50' x 20.83' x 3.54'

16 Chambers

86.1 cy Field

53.5 cy Stone



# Proposed Conditions 11-11-21

Prepared by Engineering Alliance, Inc.

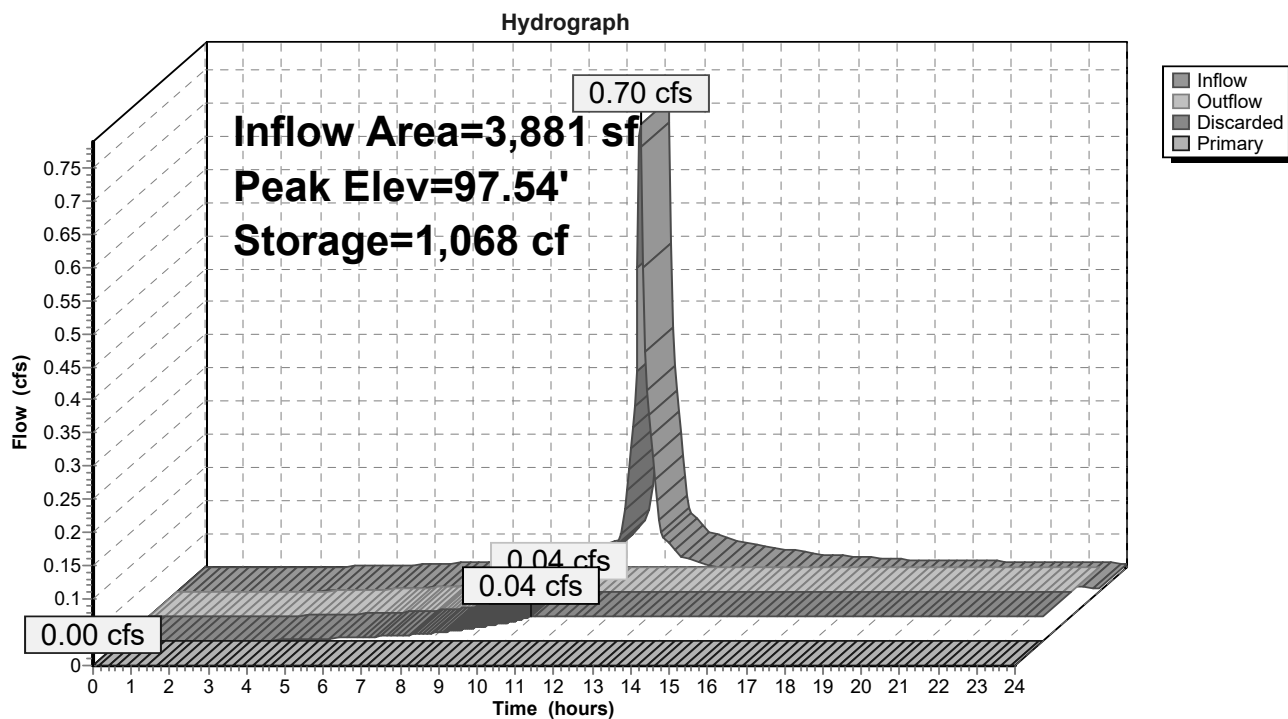
HydroCAD® 10.10-4b s/n 01924 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 100-year Rainfall=8.47"

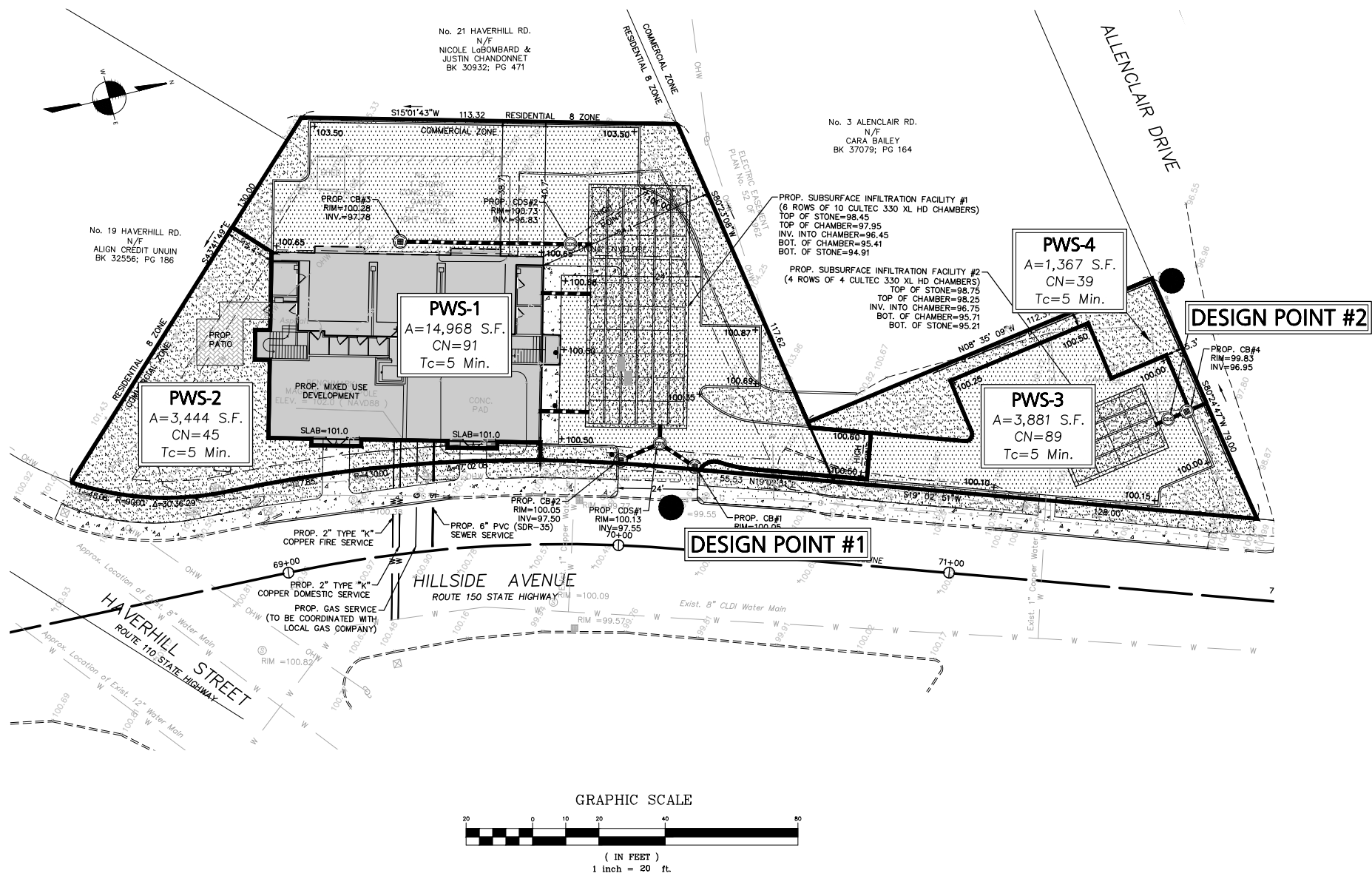
Printed 11/18/2021

Page 53

## Pond 3P: Cultec 330XL HD







LEGEND - GRADING, DRAINAGE, & UTILITY

PROPERTY LINE	
PROPOSED BUILDING	
PROPOSED DRAIN LINE	
PROPOSED CATCH BASIN	
PROPOSED CDS UNIT W/GRATE	
PROPOSED DRAIN MANHOLE	
PROPOSED CDS UNIT	
PROPOSED SEWER LINE	
PROPOSED SEWER MANHOLE	
PROPOSED PUMP CHAMBER	
PROPOSED GAS TRAP	
PROPOSED WATER LINE	
PROPOSED GAS LINE	
PROPOSED ELECTRIC LINE	
PROPOSED TRANSFORMER	
PROPOSED CONTOUR	
PROPOSED SPOT SHOT	

PREPARED BY:

**Engineering Alliance, Inc.**  
Civil Engineering & Land Planning Consultants  
194 Central Street  
Saugus, MA 01906  
Tel: (781) 231-1349  
Fax: (781) 417-0020

PROJECT:

**Site Development Plan**  
39 & 41 Hillside Avenue  
Amesbury, Massachusetts

PROJECT #: 21-76801  
SCALE: AS NOTED  
DESIGN BY: Max Friedman

DATE: October 26, 2021  
DWG FILE NAME: 21-76801.dwg  
CHECKED BY: Richard A. Salvo P.E.

Professional Engineer for  
Engineering Alliance, Inc.

OWNER:

**Angiolillo Management Group Inc.**  
99 Walnut Street  
Saugus, MA 01906

DWG. NO. **PWS**

DRAWING TITLE:  
**Proposed Watershed Plan**

DESCRIPTION OF REVISION

DATE









# **BEST MANAGEMENT PRACTICES OPERATIONS AND MAINTENANCE PLAN**

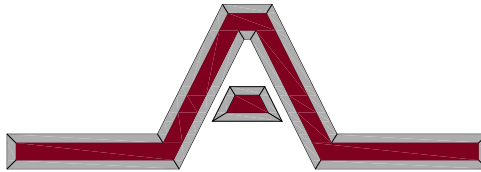
for the  
**Proposed Mixed-Use Development**

located at  
**39 & 41 Hillside Avenue  
(Tax Map 76 Lots 60 & 61)  
Amesbury, Massachusetts**

*Submitted to:*  
**City of Amesbury  
City Hall  
62 Friend Street  
Amesbury, MA 01913**

*Prepared for:*  
**Angiolillo Management Group Inc.  
99 Walnut Street  
Saugus, MA 01906**

*Prepared by:*



**Engineering Alliance, Inc.**

Civil Engineering & Land Planning Consultants	
194 Central Street	1950 Lafayette Road
Saugus, MA 01906	Portsmouth, NH 03801
Tel: (781) 231-1349	Tel: (603) 610-7100
Fax: (781) 417-0020	Fax: (603) 610-7101

**June 7, 2021**

## **BEST MANAGEMENT PRACTICES OPERATIONS AND MAINTENANCE PLAN**

A Best Management Practices Operations and Maintenance Plan is summarized below and will be incorporated into the construction documents for this project.

In accordance with the Storm Water Management Regulations issued by the Department of Environmental Protection (DEP), Engineering Alliance, Inc. has prepared the following best management practices maintenance plan for the proposed development located at 39 & 41 Hillside Avenue (Tax Map 76 Lots 60 & 61) in Amesbury, Massachusetts. The following information is broken into three sections: Construction Activities, Maintenance Budget, and Post Development Operation & Maintenance.

### **Basic Information**

Owner: Angiolillo Management Group Inc.  
99 Walnut Street  
Saugus, MA 01906

### **Section 1 - Construction Activities**

1. Contact the Amesbury Planning Department at least three (3) days prior to start of construction.
2. Install haybales and silt fence as required and silt sacs in existing catch basins in close proximity to the work to prevent sediment from entering the closed drainage system and down gradient resource areas.
3. The contractor shall only disturb the minimum area necessary.
4. The entire project area shall be swept upon completion of construction and prior to removal of the erosion control devices.

### **Section 2 – Maintenance Budget**

It is anticipated that maintenance will be required for both the temporary and permanent storm water controls. A budget shall be set aside by the contractor/owner as follows:

Construction Activities: A sum of \$2,500 shall be set aside to periodically replace temporary stormwater measures throughout all construction activities.

Post Development Construction Activities: A compounding annual budget of \$500 shall be set aside to maintain and/or replace the subsurface infiltration facilities and all other stormwater infrastructure as necessary.

### **Section 3 – Post Development Operation & Maintenance**

1. Paved Areas (Bituminous Concrete) - Paved areas shall be swept by street sweepers periodically during dry weather to remove excess sediments, reducing the amount of sediments that the drainage system will have to remove from the runoff. Salt for de-icing on the paved areas during the winter months should be limited as much as possible, as this will reduce the need for removal and treatment. Sand containing the minimum amount of calcium chloride (or approved equivalent) needed for handling may be applied as part of the routine winter maintenance activities. **At a minimum all paved areas must be swept two times annually: once in the fall and once in the spring.**
2. Cultec Sub-Surface Infiltration Facilities – Cultec Subsurface Infiltration facilities are equipped with an inspection port in each row. When the lid is removed, a screw in-plug will be exposed. Remove the plug and measure depth of sediment. If the sediment exceeds 3 inches in depth, the row should be cleaned with high pressure water through a culvert cleaning nozzle. Inlets should be periodically maintained to prevent clogging and maintain infiltration capacity.

After installation, the system should be inspected immediately after the first storm event exceeding 1' in depth to ensure proper functionality. The system should otherwise be inspected at a minimum of two times annually: one in the fall and once in the spring.

3. Catch Basins – Catch basins shall be inspected monthly for the initial twelve-month period following the completion of the construction of the paved areas. Debris shall be removed from the catch basin grates, sumps and outlet pipes and disposed of in compliance with local, state and federal guidelines.

Upon a period beginning twelve months after the completion of the site, all catch basins shall be inspected and maintained twice annually, once in April and once in November. Debris shall be removed from the catch basin grates, sumps and outlet pipes and disposed of in compliance with local, state and federal guidelines.

4. Contech CDS Units (Water Quality Manholes): Contech CDS units with manhole cover should be maintained bi-annually, after a large rain event, and when sediment levels exceed maintenance volumes, as required by the manufacturer. At a minimum, water quality manhole shall be serviced every spring and fall.
5. Snow removal and storage - Plowed snow shall be placed in the pervious area located between the parking area and property line, where it can slowly infiltrate. Sediments shall be removed from this area every spring. The storage of snow is **not** permitted in any Storm Water Facilities. When the amount of snow exceeds the capacity of the snow storage areas, it shall be removed from the site and disposed of properly immediately after each storm at the owner's expense. Refer to the proposed site plan for designated snow storage areas.
6. Pesticides, Herbicides, and Fertilizers - Pesticides and herbicides shall not be used within the limits of the 100-foot buffer zone to any wetland resource areas as defined under 310 CMR 10.00. In addition, fertilizers that are used within this zone should be restricted to the use of organic fertilizers only.
7. Maintenance Responsibilities - All post construction maintenance activities should be documented and kept on file and made available to the City of Amesbury upon request. All post construction maintenance activities shall run with the title of the property.



**NOAA Atlas 14, Volume 10, Version 3**  
**Location name: Amesbury, Massachusetts, USA\***  
**Latitude: 42.8454°, Longitude: -70.9371°**  
**Elevation: 100.37 ft\*\***  
 \* source: ESRI Maps  
 \*\* source: USGS



### POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sandra Pavlovic, Michael St. Laurent, Carl Trypaluk, Dale Unruh, Orlan Wilhite

NOAA, National Weather Service, Silver Spring, Maryland

[PF tabular](#) | [PF graphical](#) | [Maps & aeriels](#)

### PF tabular

PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches) <sup>1</sup>										
Duration	Average recurrence interval (years)									
	1	2	5	10	25	50	100	200	500	1000
5-min	0.315 (0.253-0.393)	0.377 (0.302-0.471)	0.479 (0.383-0.601)	0.563 (0.446-0.709)	0.679 (0.518-0.890)	0.766 (0.572-1.02)	0.857 (0.618-1.19)	0.958 (0.652-1.35)	1.10 (0.717-1.61)	1.22 (0.772-1.81)
10-min	0.446 (0.358-0.557)	0.534 (0.428-0.668)	0.678 (0.541-0.850)	0.797 (0.632-1.00)	0.961 (0.734-1.26)	1.09 (0.809-1.45)	1.21 (0.875-1.68)	1.36 (0.922-1.92)	1.56 (1.02-2.28)	1.73 (1.09-2.57)
15-min	0.525 (0.421-0.655)	0.628 (0.504-0.785)	0.797 (0.637-1.00)	0.938 (0.744-1.18)	1.13 (0.864-1.48)	1.28 (0.953-1.71)	1.43 (1.03-1.98)	1.60 (1.09-2.26)	1.84 (1.20-2.68)	2.03 (1.29-3.02)
30-min	0.720 (0.578-0.899)	0.862 (0.691-1.08)	1.09 (0.873-1.37)	1.29 (1.02-1.62)	1.55 (1.19-2.03)	1.75 (1.31-2.34)	1.96 (1.41-2.71)	2.19 (1.49-3.09)	2.52 (1.64-3.68)	2.79 (1.76-4.14)
60-min	0.915 (0.734-1.14)	1.10 (0.878-1.37)	1.39 (1.11-1.74)	1.63 (1.30-2.06)	1.97 (1.51-2.59)	2.23 (1.66-2.98)	2.49 (1.79-3.44)	2.78 (1.89-3.93)	3.20 (2.08-4.67)	3.54 (2.24-5.26)
2-hr	1.20 (0.969-1.49)	1.44 (1.17-1.79)	1.84 (1.48-2.30)	2.17 (1.74-2.73)	2.63 (2.03-3.45)	2.97 (2.24-3.97)	3.34 (2.44-4.63)	3.77 (2.57-5.30)	4.42 (2.88-6.41)	4.97 (3.16-7.35)
3-hr	1.40 (1.14-1.73)	1.69 (1.37-2.09)	2.17 (1.75-2.69)	2.56 (2.06-3.20)	3.11 (2.40-4.06)	3.51 (2.66-4.69)	3.95 (2.90-5.48)	4.48 (3.06-6.27)	5.29 (3.46-7.65)	5.99 (3.81-8.82)
6-hr	1.81 (1.48-2.23)	2.20 (1.80-2.71)	2.84 (2.31-3.50)	3.36 (2.71-4.17)	4.09 (3.19-5.31)	4.62 (3.52-6.14)	5.21 (3.85-7.20)	5.93 (4.06-8.24)	7.04 (4.61-10.1)	8.00 (5.10-11.7)
12-hr	2.30 (1.89-2.80)	2.81 (2.31-3.43)	3.64 (2.98-4.46)	4.33 (3.52-5.33)	5.28 (4.14-6.81)	5.98 (4.58-7.89)	6.74 (5.01-9.26)	7.68 (5.28-10.6)	9.12 (6.00-13.0)	10.4 (6.63-15.0)
24-hr	2.72 (2.26-3.30)	3.38 (2.80-4.10)	4.46 (3.67-5.42)	5.35 (4.38-6.54)	6.58 (5.19-8.45)	7.48 (5.77-9.83)	8.47 (6.34-11.6)	9.71 (6.70-13.3)	11.6 (7.68-16.5)	13.3 (8.56-19.2)
2-day	3.04 (2.54-3.66)	3.86 (3.22-4.65)	5.20 (4.31-6.28)	6.30 (5.19-7.66)	7.83 (6.23-10.0)	8.94 (6.96-11.7)	10.2 (7.71-14.0)	11.8 (8.16-16.1)	14.4 (9.51-20.3)	16.7 (10.7-23.9)
3-day	3.32 (2.78-3.98)	4.19 (3.51-5.03)	5.62 (4.68-6.77)	6.81 (5.63-8.24)	8.45 (6.75-10.8)	9.64 (7.53-12.6)	11.0 (8.34-15.0)	12.7 (8.82-17.3)	15.5 (10.3-21.8)	18.1 (11.7-25.8)
4-day	3.58 (3.01-4.29)	4.49 (3.76-5.37)	5.96 (4.98-7.16)	7.18 (5.96-8.67)	8.86 (7.10-11.3)	10.1 (7.90-13.2)	11.5 (8.73-15.6)	13.3 (9.22-18.0)	16.2 (10.7-22.6)	18.8 (12.1-26.7)
7-day	4.34 (3.67-5.16)	5.27 (4.45-6.27)	6.79 (5.71-8.11)	8.05 (6.72-9.67)	9.79 (7.88-12.4)	11.1 (8.70-14.3)	12.5 (9.52-16.9)	14.3 (10.00-19.3)	17.3 (11.5-24.1)	20.0 (12.9-28.3)
10-day	5.04 (4.27-5.97)	5.99 (5.08-7.11)	7.55 (6.37-8.99)	8.85 (7.40-10.6)	10.6 (8.57-13.3)	11.9 (9.40-15.3)	13.4 (10.2-17.9)	15.2 (10.7-20.4)	18.2 (12.1-25.2)	20.8 (13.5-29.3)
20-day	7.04 (6.02-8.29)	8.09 (6.90-9.53)	9.81 (8.33-11.6)	11.2 (9.47-13.3)	13.2 (10.7-16.3)	14.7 (11.5-18.5)	16.2 (12.3-21.2)	18.0 (12.7-24.0)	20.7 (13.9-28.4)	22.9 (14.9-32.0)
30-day	8.71 (7.47-10.2)	9.84 (8.43-11.5)	11.7 (9.96-13.8)	13.2 (11.2-15.6)	15.3 (12.4-18.8)	16.9 (13.3-21.1)	18.6 (14.0-23.9)	20.3 (14.4-26.9)	22.7 (15.3-31.1)	24.6 (16.1-34.4)
45-day	10.8 (9.32-12.6)	12.0 (10.4-14.1)	14.0 (12.0-16.4)	15.7 (13.3-18.5)	18.0 (14.6-21.8)	19.7 (15.5-24.4)	21.5 (16.1-27.3)	23.2 (16.5-30.5)	25.4 (17.2-34.6)	27.0 (17.6-37.5)
60-day	12.6 (10.9-14.7)	13.9 (12.0-16.2)	16.0 (13.7-18.7)	17.7 (15.1-20.8)	20.1 (16.4-24.4)	22.0 (17.4-27.1)	23.8 (17.9-30.1)	25.6 (18.2-33.5)	27.7 (18.8-37.5)	29.2 (19.1-40.4)

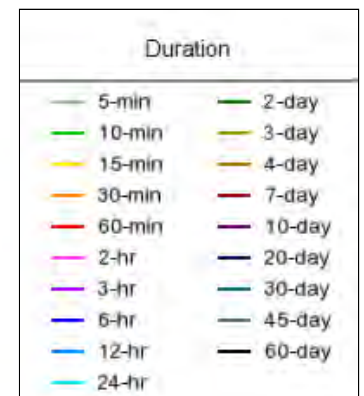
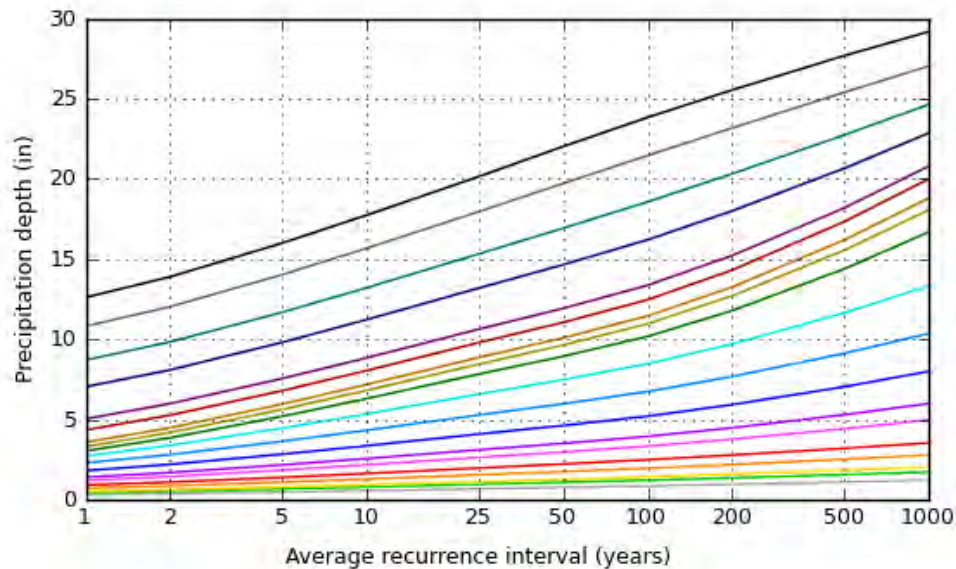
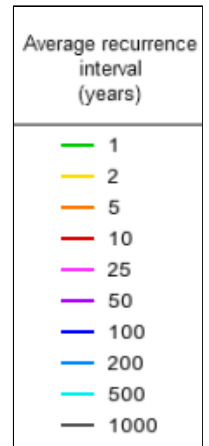
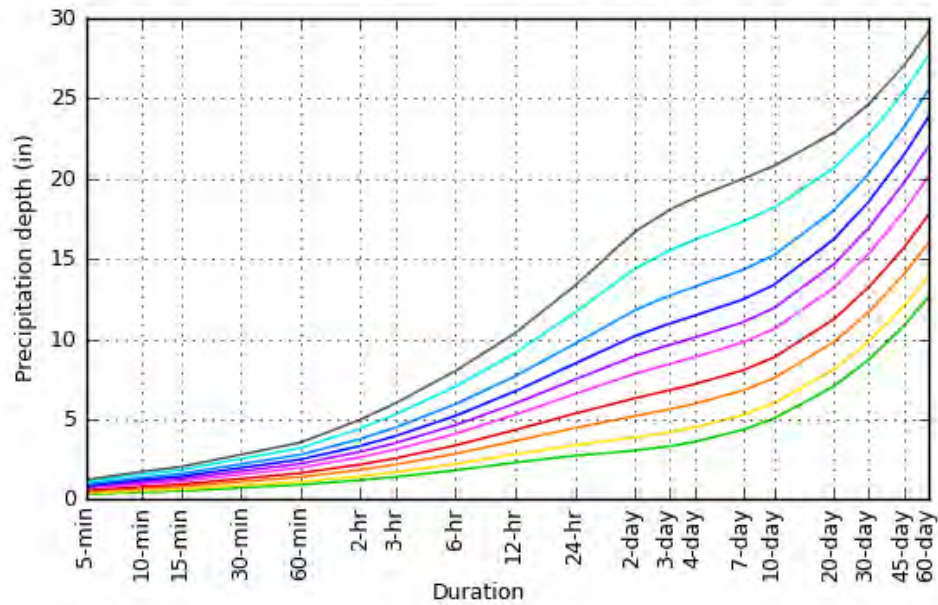
<sup>1</sup> Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS). Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values. Please refer to NOAA Atlas 14 document for more information.

[Back to Top](#)

### PF graphical

## PDS-based depth-duration-frequency (DDF) curves

Latitude: 42.8454°, Longitude: -70.9371°



NOAA Atlas 14, Volume 10, Version 3

Created (GMT): Fri Jun 4 17:33:11 2021

[Back to Top](#)**Maps & aerals****Small scale terrain**



Large scale terrain

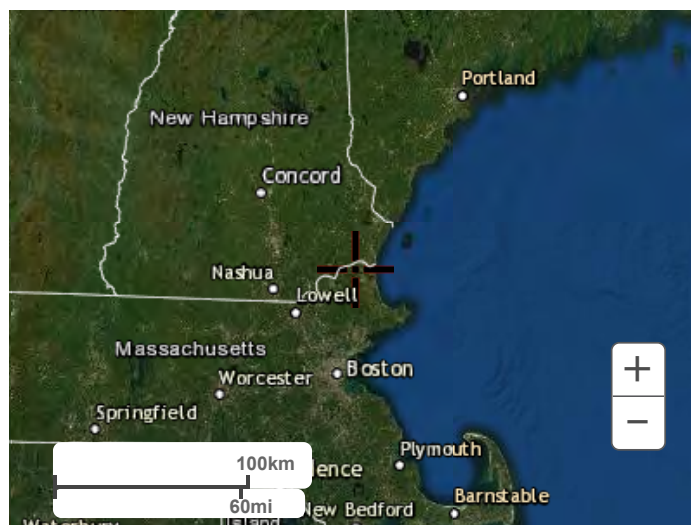


Large scale map



Large scale aerial





[Back to Top](#)

---

[US Department of Commerce](#)  
[National Oceanic and Atmospheric Administration](#)  
[National Weather Service](#)  
[National Water Center](#)  
1325 East West Highway  
Silver Spring, MD 20910  
Questions?: [HDSC.Questions@noaa.gov](mailto:HDSC.Questions@noaa.gov)

[Disclaimer](#)